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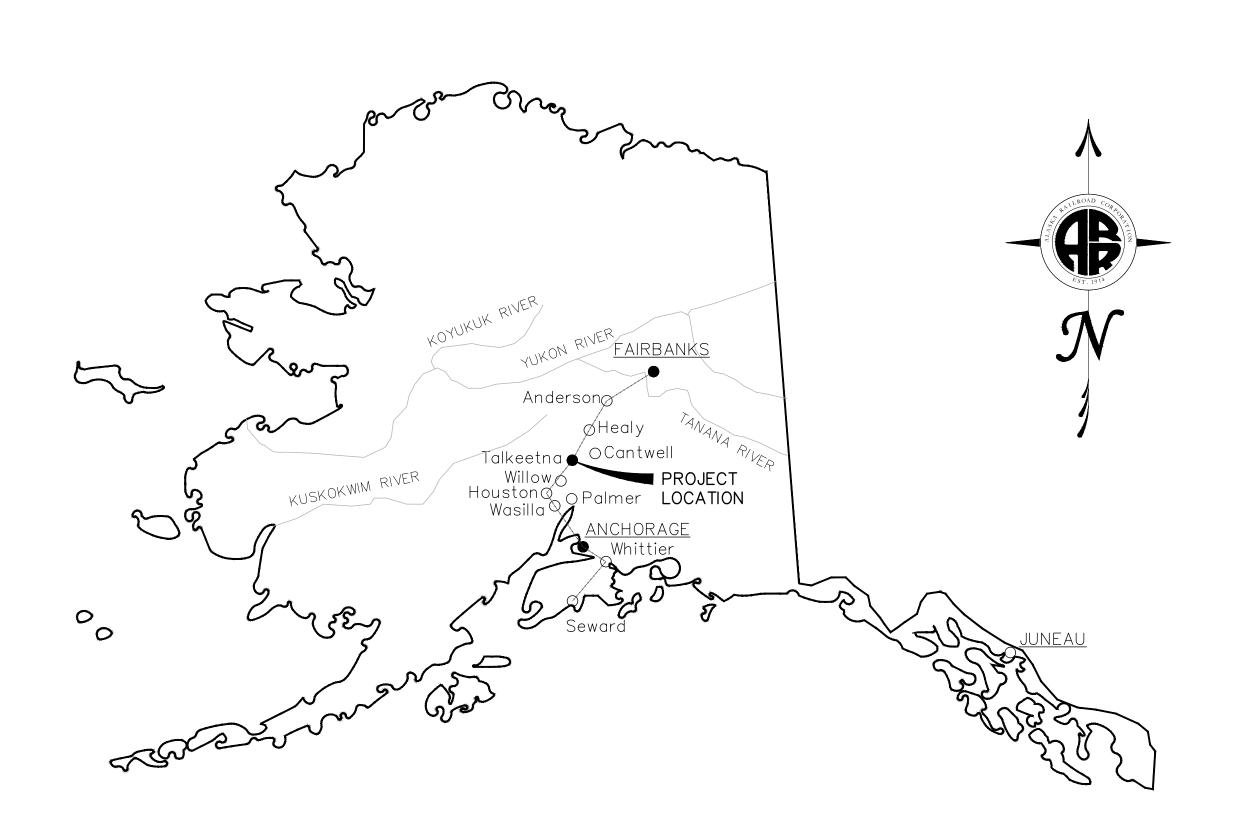
ALASKA RAILROAD CORPORATION

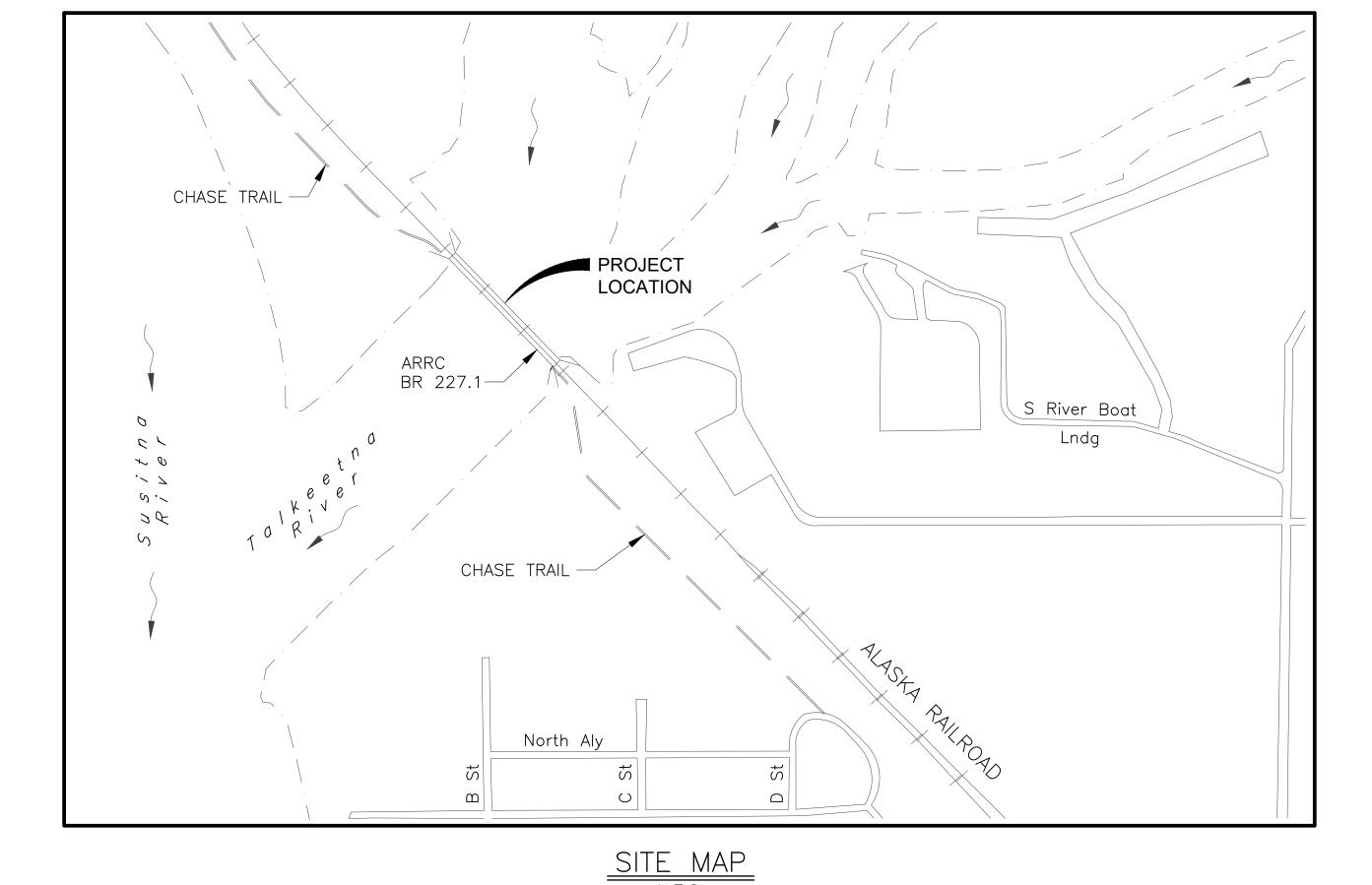
CAPITAL PROJECTS

P.O. BOX 107500, ANCHORAGE, ALASKA 99510-7500

TALKEETNA RIVER BRIDGE 227.1 REHABILITATION ISSUED FOR BID PLAN SET

DECEMBER 2023





VICINITY MAP

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STANDARD DRAWING REFERENCES:

ARRC STANDARD BALLAST AND TRACK PLANS — CURRENT VERSION

ALASKA DOT STANDARD PLANS F—01.04 — CHAIN LINK FENCE

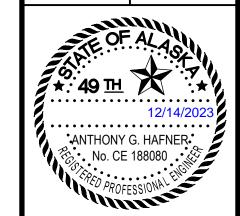
AS-BUILT DRAWING REFERENCES:

ARRC — BRIDGE NO. 227.1 ~ TALKEETNA RIVER — GENERAL ELEVATION, FOUNDATION — NOV. 1925

ARRC — BRIDGE NO. 227.1 ~ TALKEETNA RIVER — MASONRY PLAN & ERECTION DIAGRAM — SHEETS E1/E2 — NOV.1924

ARRC — BRIDGE NO. 227.1 ~ TALKEETNA RIVER — TRUSS PLANS — SHEETS 1 THRU 7 — JAN. 1925

DESIGNED BY:	АН
CHECKED BY:	МН
DRAFTED BY:	VS



HDR ENGINEERING, INC.
582 E. 36TH AVE, SUITE 500
ANCHORAGE, AK 99503-4169
(907) 644-2000
LICENSE #: AECC569

	SR2 F 36TH AVE SHITE 50	ANCHORAGE, AK 99503-416 (907) 644-2000	
CAPITAL PROJECTS	P.O. BOX 107500 ANCHORAGE, ALASKA 99510—7500	RIDGE 227.1 REHABILITATION	
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AFE NO. 12259
YEAR 2023

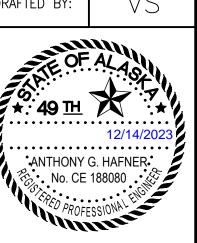
SHEET 1 OF 28

KDOT & PF	ALASKA DEPARTMENT OF TRANSPORTATION	LEGEND
PPROX.	& PUBLIC FACILITIES APPROXIMATE	GENERAL
REMA	AMERICAN RAILWAY ENGINEERING AND MAINTENANCE OF WAY ASSOCIATION	
RRC	ALASKA RAILROAD CORPORATION	EXISTING PROPOSED
STM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	
VS	AMERICAN WELDING SOCIETY	BUILDING / / /
1P	BEST MANAGEMENT PRACTICE	
)	BRIDGE	CENTERLINE — — — — — — — — — —
	CENTERLINE	
-C RSI	CENTER—TO—CENTER CONCRETE REINFORCING STEEL INSTITUTE	
, SI	DIAMETER	EXISTING TRAIL ======
OR EXP	EXPANSION	
EV. OR EL.	ELEVATION	EDGE OF GRAVEL
OR FIX	FIXED	
ALV.	GALVANIZED	EDGE OF PAVEMENT
2	GRADE	LUCE OF FAVEIVIENT
ORIZ.	HORIZONTAL	
S.	HIGH STRENGTH	CHAIN LINK FENCE ———————————————————————————————————
-	LINEAR FEET	
N.	MINIMUM	RAILROAD BRIDGE
<	MARK	
	MILEPOST	$ au \cap \lambda \cap ec{ec}$
)T	NON-DESTRUCTIVE TEST	TRACK &
).	NUMBER OF THE RESULT OF THE RE	
) -	OPEN DECK	RIGHT OF WAY
	OVERHEAD ELECTRICAL LINE	
⊣W ○	ORDINARY HIGH WATER	RIP RAP
-0 CF	OUT-TO-OUT POUNDS PER CUBIC FOOT	
√ Γ -	PLATE	
- SF	POUNDS PER SQUARE FOOT	SIGN
SI	POUNDS PER SQUARE INCH	
ΓFE	POLYTETRAFLUOROETHYLENE	
	RADIUS	
/w or row	RIGHT OF WAY	UTILITIES
_H	SHORT LEG HORIZONTAL	<u></u>
	SHORT LEG VERTICAL	EVIOTIVIO
PA	SPACE OR SPACING	EXISTING PROPOSED
SHC	STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTIONS	
SPC	STEEL STRUCTURES PAINTING COUNCIL	OVERHEAD POWEROEOEOE
Ā.	STATION	
/XXX	TOP OF XXX (T/TIE, T/CAP, ETC.)	UNDERGROUND ————————————————————————————————————
BD.	TO BE DETERMINED (DESIGNED)	ELECTRIC ————————————————————————————————————
/P.	TYPICAL	ELECTRIC IN
ERT.	VERTICAL	CONDUIT ————————————————————————————————————
.		
		TOPOGRAPHY
OLE LE	GEND:	
SHOP B		EXISTING PROPOSED
		EXISTING PROPOSED
FIELD BO		
		CONTOURS 100' 100'
		FLOW DIRECTION -
		FLOW DIRECTION

DESIGNED BY: AH

CHECKED BY: MH

DRAFTED BY: VS



HDR ENGINEERING, INC. 582 E. 36TH AVE, SUITE 500 ANCHORAGE, AK 99503-4169 (907) 644-2000 LICENSE #: AECC569

582 E. 36TH AVE, SUITE 500 ANCHORAGE, AK 99503-4169 (907) 644-2000

CAPITAL PROJECTS
P.O. BOX 107500
ANCHORAGE, ALASKA 99510-7500
TALKEETNA RIVER BRIDGE 227.1 REHABILITATION
ET TITLE:
LEGEND AND ABBREVIATIONS

AFE NO. 12259

YEAR 2023

SHEET 2 OF 28

ESTIMATED QUANTITY NOTES:

1. QUANTITIES ARE ESTIMATED, CONTRACTOR TO FIELD VERIFY ALL QUANTITIES BASED ON SITE CONDITIONS AND DETAILS NOTED ON PLANS PRIOR TO ORDERING MATERIAL OR STARTING CONSTRUCTION.

CONSTRUCTION (SSHC) PAY ITEM NUMBERS ARE PROVIDED FOR REFERENCE ONLY.

2. FOR DESCRIPTION OF PAY ITEMS, REFER TO PROJECT SPECIFICATIONS.

3. REFERENCES TO 2020 AK DOT&PF STANDARD SPECIFICATIONS FOR HIGHWAY

REFER TO PROJÈCT SPECIFICATIONS FOR APPLICABLE PORTIONS OF WORK.

	ESTIMATE OF QUANTITIES		
PAY ITEM	ITEM DESCRIPTION	PAY UNIT	TOTAL QUANTIT
201.0009.1	CLEARING AND GRUBBING	ACRE	0.25
202.0023.1	REMOVAL OF EXISTING STRUCTURES	LS	1
501.0007.1	PRECAST CONCRETE RISER BLOCK	EA	6
504.0001.1	STRUCTURAL STEEL, TRUSS REHABILITATION	LS	1
504.0001.2	STRUCTURAL STEEL, TRAINMAN'S WALKWAY	LS	1
504.0001.3	STRUCTURAL STEEL, TRUSS MEMBER SPLICE	CS	ALL REQ'D
506.0001.1	TREATED TIMBER, TRAINMAN'S WALKWAY	LS	1
507.0002.1	PEDESTRIAN RAILING	LF	880
516.0002.1	BEARINGS, TRUSS	EA	8
607.0003.1	CHAIN LINK FENCE 8'-0" TALL	LF	200
640.0001.1	MOBILIZATION AND DEMOBILIZATION	LS	1
641.0002.1	TEMPORARY EROSION, SEDIMENT, AND POLLUTION CONTROL AND ADMINISTRATION	LS	1
643.0001.1	TRAFFIC CONTROL, ADMINISTRATION AND MAINTENANCE	CDAY	90
643.0001.3	PERMANENT CONSTRUCTION SIGNS	LS	1
646.0001.1	CPM SCHEDULING	LS	1
647.0001.1	AS-BUILT DRAWINGS	LS	1

ITEM 202.0023.1 - REMOVAL OF EXISTING STRUCTURES				
DESCRIPTION	UNIT	QUANTITY		
REMOVAL OF EXISTING TRAINMAN WALKWAY, WALKWAY TIMBER ABUTMENTS AND TIMBER RETAINING WALLS	LF	431		
REMOVAL OF EXISTING CHAIN LINK FENCE	LF	200		
REMOVAL OF EXISTING TRUSS BEARINGS	EA	8		
REMOVAL OF EXISTING STRINGER FLANGE ANGLES	EA	56		
REMOVAL OF EXISTING TRUSS END FLOORBEAM FLANGE ANGLES	EA	8		

ITEM 501.0007.1 - PRECAST CONCRETE RISER BLOCK (NOTE 1)				
DESCRIPTION	UNIT	QUANTITY		
PRECAST CONCRETE RISER BLOCK MK CRB1	EA	4		
PRECAST CONCRETE RISER BLOCK MK CRB2	EA	2		
DRILL AND BOND ANCHOR ROD	EA	32		
CONCRETE REPAIR AND GROUT PADS	CY	1.5		
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF STEEL FASTENERS REQUIRED	MATERIAL	AND		

ITEM 504.0001.1 - STRUCTURAL STEEL, TRUSS REHABILITATION (NOTE 1)				
DESCRIPTION	UNIT	QUANTITY		
REPLACE EXISTING RIVET WITH H.S. BOLTS	EA	10,854		
NON-DESTRUCTIVE TESTING OF EXISTING WELD REMOVALS	LF	50		
STRUCTURAL STEEL, COVER PLATES	LB	4,193		
STRUCTURAL STEEL, STRINGER FLANGE ANGLES	LB	53,872		
STRUCTURAL STEEL, END FLOORBEAM FLANGE ANGLES	LB	3,088		
TOTAL STRUCTURAL STEEL	LB	61,153		
TEMPORARY ERECTION H.S. BOLTS (NOTE 2)	EA	3,885		
PERMANENT H.S. BOLTS (NOTE 2)	EA	12,246		
TOTAL H.S BOLTS	EA	16,131		
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF STEEL FASTENERS REQUIRED	MATERIAL	AND		
NOTE 2: BOLT QUANTITIES INCLUDE 5% ADDITIONAL				

ITEM 504.0001.2 — STRUCTURAL STEEL, TRAINMAN'S	WALKWA	Y (NOTE 1)
DESCRIPTION	UNIT	QUANTITY
STRUCTURAL STEEL, CONNECTION ANGLES	LB	2,911
STRUCTURAL STEEL, BRACKETS	LB	2,214
STRUCTURAL STEEL, STRINGERS	LB	24,822
STRUCTURAL STEEL, CROSSBEAMS	LB	3,925
STRUCTURAL STEEL, PLATES	LB	997
TOTAL STRUCTURAL STEEL	LB	34,869
PERMANENT H.S. BOLTS (NOTE 2)	EA	1,474
ELASTOMERIC PAD	EA	32
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF STEEL FASTENERS REQUIRED	MATERIAL	AND
NOTE 2: BOLT QUANTITIES INCLUDE 5% ADDITIONAL		

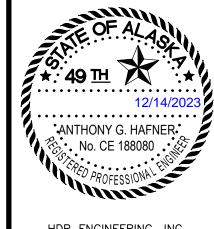
ITEM 504.0001.3 - STRUCTURAL STEEL, TRUSS MEMI	BER SPLIC	CE (NOTE 1)
DESCRIPTION	UNIT	QUANTITY
REPLACE EXISTING RIVET WITH H.S. BOLTS (PER SPLICE)	EA	159
STRUCTURAL STEEL, COVER PLATES (PER SPLICE)	LB	1,228
PERMANENT H.S. BOLTS (NOTE 2) (PER SPLICE)	EA	159
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF STEEL FASTENERS REQUIRED	MATERIAL	AND
NOTE 2: BOLT QUANTITIES INCLUDE 5% ADDITIONAL		

ITEM 506.0001.1 - TREATED TIMBER, TRAINMAN'S	WALKWAY	(NOTE 1)
DESCRIPTION	UNIT	QUANTITY
3x12 (2.5"x11.25" ROUGH SAWN) COPPER NAPTHENATE TREATED TIMBER DECKING	SF	1,626
2x12 (1.5"x11.25" ROUGH SAWN) COPPER NAPTHENATE TREATED TIMBER DECKING	SF	1,770
CARRIAGE BOLTS (NOTE 2)	EA	700
TIMBER LAG SCREWS (NOTE 2)	EA	1,365
8x10 (ROUGH SAWN) COPPER NAPTHENATE TREATED TIMBER STRINGERS, 18'-0" LONG	EA	10
12x12 (ROUGH SAWN) COPPER NAPTHENATE TREATED TIMBER MUD SILLS, 5'-0" LONG	EA	4
#6 REBAR DOWELS, 4'-0" LONG	EA	12
TIMBER STRINGER TIE BOLTS & HARDWARE	EA	10
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF FASTE	NERS REQU	JIRED
NOTE 2: BOLT AND SCREW QUANTITIES INCLUDE 5% ADDITIONA	AL.	

ITEM 507.0002.1 — PEDESTRIAN RAILING			
DESCRIPTION	UNIT	QUANTITY	
STEEL TUBE PEDESTRIAN RAILING, GALVANIZED		880	
CONCRETE FILLED STEEL BOLLARD		4	

ITEM 516.0002.1 - BEARINGS, TRUSS (NOTE 1)	
DESCRIPTION	UNIT	QUANTITY
EXPANSION BEARING ASSEMBLY	EA	4
FIXED BEARING ASSEMBLY	EA	4
NOTE 1: SEE BILL OF MATERIAL FOR DETAILED LIST OF STEEL FASTENERS REQUIRED	MATERIAL	AND

DESIGNED BY:	MH
CHECKED BY:	МН
DRAFTED BY:	VS



HDR ENGINEERING, INC. 582 E. 36TH AVE, SUITE 500 ANCHORAGE, AK 99503-4169 (907) 644-2000 LICENSE #: AECC569

582 E. 36TH AVE, SUITE 500 ANCHORAGE, AK 99503-4169 (907) 644-2000

F.O. BOX 107500

AD ————
ANCHORAGE, ALASKA 99510—7

ETNA RIVER BRIDGE 227.1 REHABILITATION

AFE NO. 12259

YEAR 2023

SHEET 3 OF 28

DATE	TIME	SCALE	PUBLISHED CTB
12/14/2023	11:03 AM	AS NOTED	ARRC_CTB_2023.CTB

	BILL OF MATERIAL — BEARING REPLACEMENT (ITEMS 501.0007.1 PRECAST CONCRETE RISER BLOCK AND 516.0002.1 BEARINGS, TRUSS)								
LINE	QUANTITY	UNIT	DESCRIPTION	MARK	SIZE	LENGTH	LIFTING WEIGHT (LBS)	REMARKS	
1	4	EA.	FIXED BEARING MASONRY BASE PLATE	MBP1	5¾"×2'-9"	4'-2"	2,370	ASTM A588 GR. 50W, SEE DETAIL ON SHEET 18	
2	4	EA.	EXPANSION BEARING BASE PLATE	BP2	2"x2'-9"	4'-2"	975	ASTM A588 GR. 50W, SEE DETAIL ON SHEET 18	
3	4	EA.	EXPANSION BEARING MASONRY PLATE	MP2	3½"×2'-9"	4'-2"	1,638	ASTM A588 GR. 50W, SEE DETAIL ON SHEET 18	
4	8	EA.	ELASTOMERIC TRUSS BEARING PAD	EBP1	½"x2'-10"	4'-3"		60 DUROMETER NATURAL RUBBER, SEE DETAIL ON SHEET 18	
5	4	EA.	PRECAST CONCRETE RISER BLOCK	CRB1	6"x3'-1"	4'-9"	1,305	SEE DETAIL ON SHEET 19	
6	2	EA.	PRECAST CONCRETE RISER BLOCK	CRB2	6"x5'-0"	6'-5"	2,705	SEE DETAIL ON SHEET 19	
7	32	EA.	HIGH STRENGTH BOLTS		1½" DIA.	0'-4"			
8	32	EA.	ANCHOR ROD W/ 2~NUTS & FLAT WASHER, GALVANIZED	AR1	1½" DIA.	3'-0"		SEE DETAIL ON SHEET 18	
9	32	EA.	PLATE WASHER	PW1	½"x 5"	0'-8"	5.7	ASTM A588 GR. 50W, SEE DETAIL ON SHEET 18	
10	1	CY	CONCRETE REPAIR					SEE DETAIL ON SHEET 8	
11	0.5	CY	POLYMER GROUT PAD					SEE DETAILS ON SHEET 8	

BILL OF MATERIAL — TRUSS REHABILITATION (ITEM 504.0001.1 — STRUCTURAL STEEL, TRUSS REHABILITATION)

LINE	QUANTITY	UNIT	DESCRIPTION	MARK	SIZE	LENGTH	LIFTING WEIGHT (LBS)	REMARKS
1	2	EA.	STEEL COVER PLATE	P23	³ / ₈ "×14"	30'-11"	552	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
2	2	EA.	STEEL COVER PLATE	D2	%"×12"	39'-45/16"	1005	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
3	1	EA.	STEEL SPLICE PLATE	P21S	³ ⁄ ₈ "×14"	3'-1"	55.1	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
4	2	EA.	STEEL SPLICE PLATE	P23S	¾"×6"	3'-1"	23.6	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
5	1	EA.	STEEL SPLICE PLATE	P22S1	³ ⁄ ₈ "×14"	2'-83/8"	48.2	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
6	2	EA.	STEEL SPLICE PLATE	P22S2	½"×5¼"	2'-8%"	24.1	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
7	2	EA.	STEEL SPLICE PLATE	SSPD1	5/8"×12"	4'-0%"	104	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
8	2	EA.	STEEL SPLICE PLATE	SSPD2	5/8"×12"	4'-0½"	103	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
9	1	EA.	STEEL FILL PLATE	SFP1	½"×14"	1'-5"	8.5	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
10	1	EA.	STEEL FILL PLATE	SFP2	⅓"×14"	1'-4"	8.0	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
11	1	EA.	STEEL FILL PLATE	SFP3	½"×14"	1'-5"	33.8	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
12	2	EA.	STEEL FILL PLATE	SFP4	3/8"×31/4"	1'-1½"	4.7	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
13	1	EA.	STEEL FILL PLATE	SFP2L5	%"×14"	1'-4"	39.7	ASTM A709 GR 50, SEE DETAILS ON SHEET 1
14	2	EA.	STEEL FILL PLATE	SFPD1	½"×12"	1'-4½"	7.0	ASTM A709 GR 50, SEE DETAILS ON SHEET
15	2	EA.	STEEL FILL PLATE	SFPD2	⅓"×12"	1'-5¾"	7.6	ASTM A709 GR 50, SEE DETAILS ON SHEET
16	2	EA.	STEEL CHEESE PLATE	SCPD1	¾"×12"	2'-61/8"	76.9	ASTM A709 GR 50, SEE DETAILS ON SHEET
17	2	EA.	STEEL CHEESE PLATE	SCPD2	¾"×12"	2'-6"	76.6	ASTM A709 GR 50, SEE DETAILS ON SHEET
18	1	EA.	STEEL BRACING GUSSET PLATE	BCP1	¾"×16"	1'-5%"	30.0	ASTM A709 GR 50, SEE DETAILS ON SHEET
19	28	EA.	STRINGER FLANGE ANGLE	SFA1	L8x6x ³ / ₄	28'-57/6"	962	ASTM A709 GR 50, SEE DETAILS ON SHEET
20	28	EA.	STRINGER FLANGE ANGLE	SFA2	L8x6x3/4	28'-57/6"	962	ASTM A709 GR 50, SEE DETAILS ON SHEET
21	4	EA.	FLOORBEAM FLANGE ANGLE	FFA1	L8x6x½	16'-91/8"	386	ASTM A709 GR 50, SEE DETAILS ON SHEET
22	4	EA.	FLOORBEAM FLANGE ANGLE	FFA2	L8×6×1/2	16'-91/8"	386	ASTM A709 GR 50, SEE DETAILS ON SHEET
23	1,590	EA.	HIGH STRENGTH BOLT		%" DIA.	21/4"		ASTM F3125, A325, TYPE 3
24	2,270	EA.	HIGH STRENGTH BOLT		%" DIA.	2½"		ASTM F3125, A325, TYPE 3
25	1,030	EA.	HIGH STRENGTH BOLT		%" DIA.	23/4"		ASTM F3125, A325, TYPE 3
26	855	EA.	HIGH STRENGTH BOLT		%" DIA.	3"		ASTM F3125, A325, TYPE 3
27	25	EA.	HIGH STRENGTH BOLT		%" DIA.	31/4"		ASTM F3125, A325, TYPE 3
28	6,040	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	3½"		ASTM F3125, A325, TYPE 3
29	18	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	4"		ASTM F3125, A325, TYPE 3
30	30	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	41/4"		ASTM F3125, A325, TYPE 3
31	270	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	4½"		ASTM F3125, A325, TYPE 3
32	118	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	43/4"		ASTM F3125, A325, TYPE 3
33	273	EA.	HIGH STRENGTH BOLT — TEMPORARY		7⁄8" DIA.	23/4"		ASTM F1554, A325, TYPE 1 OR TYPE 3
34	3,528	EA.	HIGH STRENGTH BOLT — TEMPORARY		7⁄8" DIA.	3½"		ASTM F1554, A325, TYPE 1 OR TYPE 3
35	84	EA.	HIGH STRENGTH BOLT — TEMPORARY		7⁄8" DIA.	4"		ASTM F1554, A325, TYPE 1 OR TYPE 3
36	12,246	EA.	HEAVY HEX NUT		7⁄8" DIA.			ASTM A563, GRADE C3
37	12,246	 ЕА.	WASHER, FLAT, ROUND					ASTM F436, TYPE 3

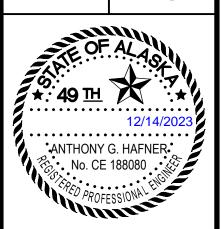
BILL OF MATERIAL — WALKWAY (ITEM 504.0001.2 — STRUCTURAL STEEL, TRAINMAN'S WALKWAY, ITEM 506.0001.1 — TREATED TIMBER, TRAINMAN'S WALKWAY AND ITEM 507.0002.1 — PEDESTRIAN RAILING)

LINE	QUANTITY	UNIT	DESCRIPTION	MARK	SIZE	LENGTH	LIFTING WEIGHT (LBS)	REMARKS
1	1	EA.	STEEL TUBE HANDRAIL PANEL	AH1		14'-3%6"	750	SEE DETAILS ON SHEET 24
2	1	EA.	STEEL TUBE HANDRAIL PANEL	AH1A		14'-3%6"	657	SEE DETAILS ON SHEET 24
3	2	EA.	STEEL TUBE HANDRAIL PANEL	AH2		21'-0%"	996	SEE DETAILS ON SHEET 24
4	32	EA.	STEEL TUBE HANDRAIL PANEL	AH3		21'-411/16"	992	SEE DETAILS ON SHEET 24
5	2	EA.	STEEL TUBE HANDRAIL PANEL	AH4		12'-11/4"	567	SEE DETAILS ON SHEET 24
6	2	EA.	STEEL TUBE HANDRAIL PANEL	AH5		12'-1¾"	569	SEE DETAILS ON SHEET 24
7	2	EA.	STEEL TUBE HANDRAIL PANEL	AH6		20'-37/16"	962	SEE DETAILS ON SHEET 25
8	1	EA.	STEEL TUBE HANDRAIL PANEL	AH7		14'-91/4"	768	SEE DETAILS ON SHEET 25
9	1	EA.	STEEL TUBE HANDRAIL PANEL	AH7A		14'-91/4"	673	SEE DETAILS ON SHEET 25
10	28	EA.	STEEL SUPPORT BEAM	SB	W8×31	28'-5%"	887	SEE DETAILS ON SHEET 23
11	58	EA.	STEEL CROSS BEAM	СВ	W6x15	4'-7½"	66.1	SEE DETAILS ON SHEET 23
12	15	EA.	STEEL WALK SUPPORT BRACKET	BR	6"x3'-7¾"	4'-5"	139	SEE DETAILS ON SHEET 23
13	1	EA.	STEEL WALK SUPPORT BRACKET	BRU5	6"×3'-7¾"	4'-5"	139	SEE DETAILS ON SHEET 23
14	2	EA.	STEEL ANGLE POST	BR2a	L4x4x ³ / ₈	3'-51/4"	33.7	SEE DETAILS ON SHEET 28
15	2	EA.	STEEL CHANNEL BEAM	BR2b	C7x9.8	4'-8"	45.8	SEE DETAILS ON SHEET 28
16	4	EA.	STEEL ANGLE BRACKET	SB1	L6x4x3/8	0'-4"	4.0	SEE DETAILS ON SHEET 28
17	2	EA.	STEEL ANGLE BRACKET	SB2	L6x4x3/8	0'-4"	4.0	SEE DETAILS ON SHEET 28
18	2	EA.	STEEL FILL PLATE	SFP6	3/8"×7"	0'-10"	7.5	SEE DETAILS ON SHEET 28
19	32	EA.	TOP BRACKET ANGLE	BRA1	L6×6×½	0'-11½"	38.9	SEE DETAILS ON SHEET 23
20	32	EA.	BOTTOM BRACKET ANGLE	BRA2	L6×6×½	1'-3"	49.2	SEE DETAILS ON SHEET 23
21	2	EA.	STEEL HANDRAIL PLATE	HP1	½"×15½"	15'-9"	416	SEE DETAILS ON SHEET 28
22	3	EA.	WALKWAY COVER PLATE		WT6x13	3'-10"	50	ASTM A709 GR. 50, GALV.
23	32	EA.	ELASTOMERIC PAD	EP1	⅓"×6"	0'-7%"		60 DUROMETER NATURAL RUBBE SEE DETAILS ON SHEET 22
24	735	EA.	CARRIAGE BOLT W/ LOCKNUT AND 2 FLAT WASHERS, GALVANIZED FOR TIMBER DECKING		½" DIA.	4"		TIMBER DECK TO CROSSBEAM
25	1,300	EA.	TIMBER LAG SCREW, EXTERIOR COATED		#15	3½"		TIMBER SURFACE PLANK TO LOWER TIMBER
26	65	EA.	TIMBER LAG SCREW, EXTERIOR COATED		½" DIA.	6"		SB1 TO MUD SILL, HANDRAIL PANEL AND PLATE TO TIMBER STRINGERS
27	257	EA.	HIGH STRENGTH BOLT		7⁄8" DIA.	4½"		ASTM F3125 A325 TYPE 3
28	5	EA.	HIGH STRENGTH BOLT		3/4" DIA.	13/4"		ASTM F3125 A325 TYPE 3
29	488	EA.	HIGH STRENGTH BOLT		3/4" DIA.	2½"		ASTM F3125 A325 TYPE 3
30	488	EA.	HIGH STRENGTH BOLT		3/4" DIA.	27/8"		ASTM F3125 A325 TYPE 3
31	236	EA.	HIGH STRENGTH BOLT		%" DIA.	3½"		ASTM F3125 A325 TYPE 3
32	981	EA.	HEAVY HEX NUT		3/4" DIA.			ASTM A563, GRADE C3
33	981	EA.	WASHER, FLAT ROUND		3/4" DIA.			ASTM F436, TYPE 3
34	493	EA.	HEAVY HEX NUT		%" DIA.			ASTM A563, GRADE C3
35	493	EA.	WASHER, FLAT ROUND		%" DIA.			ASTM F436, TYPE 3
36	13	EA.	MACHINE BOLT, GALV.		½" DIA.	0'-10"		ASTM A307
37	10	EA.	THREADED ROD, GALV.		½" DIA.	4'-5"		ASTM F1554 GRADE 36
38	33	EA.	HEX NUT, GALV.		½" DIA.			ASTM A563, GRADE C
39	33	EA.	OGEE TIMBER WASHER, GALV.		½" DIA.			
40	9	EA.	MACHINE BOLT, GALV.		1" DIA.	2'-2"		ASTM A307
41	9	EA.	HEX NUT, GALV.		1" DIA.			ASTM A563, GRADE C
42	9	EA.	PLATE WASHER, GALV.		½"×4"	0'-4"		ASTM A36

BILL OF MATERIAL -	- TRUSS MEMBER SPLICE (I	TFM 504.0001.3 -	STRUCTURAL STEFL.	TRUSS MEMBER SPLICE)
2122 01 1111 1121 1111			0111001011112	111000 11101110111101111101111111111111

LINE	QUANTITY	UNIT	DESCRIPTION	MARK	SIZE	LENGTH	LIFTING WEIGHT (LBS)	REMARKS
1	AS REQ.	EA.	STEEL COVER PLATE	LO1L	%"×16"	10'-6"	405	ASTM A709 GR 50, SEE DETAILS ON SHEET 17
2	AS REQ.	EA.	STEEL CHEESE PLATE	SCPL	¾"×16"	10'-6"	439	ASTM A709 GR 50, SEE DETAILS ON SHEET 17
3	AS REQ.	EA.	STEEL SPLICE PLATE	SSPD3	3/8"×123/4"	6'-1%"	100	ASTM A709 GR 50, SEE DETAILS ON SHEET 17
4	AS REQ.	EA.	STEEL FILL PLATE	SFPD3	1¼"×12¾"	3'-11%"	215	ASTM A709 GR 50, SEE DETAILS ON SHEET 17
5	AS REQ.	EA.	STEEL CHEESE PLATE	SCPD3	¾"×12¾"	2'-1½"	69.2	ASTM A709 GR 50, SEE DETAILS ON SHEET 17
6	55	EA.	HIGH STRENGTH BOLT		%" DIA.	23/4"		ASTM F1554, A325, TYPE 3
7	49	EA.	HIGH STRENGTH BOLT		%" DIA.	31/4"		ASTM F1554, A325, TYPE 3
8	55	EA.	HIGH STRENGTH BOLT		7⁄8" DIA,	4"		ASTM F1554, A325, TYPE 3
9	159	EA.	HEAVY HEX NUT		%" DIA.			ASTM A563, GRADE C3
10	159	EA.	WASHER, FLAT, ROUND		%" DIA.			ASTM F436, TYPE 3
NOTE:	BOLT QUA	NTITIES	INCLUDE 5% ADDITIONAL	·			·	

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EAR 2023

SHEET 4 OF 2

GENERAL NOTES:

- 1. DESIGN AND FABRICATION OF NEW TRUSS COMPONENTS SHALL BE IN ACCORDANCE WITH THE 2021 AREMA MANUAL FOR RAILWAY ENGINEERING CHAPTER 8, CONCRETE STRUCTURES AND FOUNDATIONS, AND CHAPTER 15, STEEL STRUCTURES.
- 2. DESIGN LIVE LOADING FOR NEW TRUSS COMPONENTS IS A 286k CONSIST WITH IMPACT. FATIGUE IS ALSO CONSIDERED.
- 3. DESIGN LIVE LOADING FOR TRAINMAN'S WALK IS 85 PSF UNIFORM LOAD OR ATV CONSISTING OF 4-500 LBS WHEEL LOADS WITH AN AXLE SPACING OF 4'-0". SNOW LOAD IS 24" OF COMPACTED SNOW.
- 4. CONTRACTOR TO PROVIDE A TEMPORARY EROSION AND SETTLEMENT CONTROL PLAN IN ACCORDANCE WITH THE ENVIRONMENTAL PERMITS AND STATE OF ALASKA BMPS.
- 5. CONTRACTOR MAY IN LIEU OF COVER PLATING MEMBERS AS PROPOSED REPLACE THOSE MEMBERS IN KIND, SUBJECT TO REVIEW AND APPROVAL FROM ARRC.
- 6. CONTRACTOR TO PROVIDE LEAD PAINT CONTAINMENT AND DISPOSAL PLAN IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.
- 7. ALL PLAN DIMENSIONS AND STEEL TRUSS REHAB DETAILS ARE BASED ON AVAILABLE AS-BUILTS. CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL DIMENSIONS PRIOR TO ORDERING MATERIALS. IF ACTUAL FIELD DIMENSIONS DO NOT MATCH THESE PLANS NOTIFY ARRC AND THE BRIDGE ENGINEER PRIOR TO ORDERING MATERIALS.
- 8. CONTRACTOR TO RETURN SITE TO EXISTING OR BETTER CONDITIONS AT CONCLUSION OF CONSTRUCTION PRIOR TO DEMOBILIZATION.

HANDRAIL AND WALKWAY NOTES:

- 1. HSS SQUARE TUBE SHALL CONFORM TO ASTM A500 GR B.
- 2. ALL WALKWAY STRUCTURAL STEEL SHALL CONFORM TO ASTM DESIGNATION A709 GRADE 50W EXCEPT AS NOTED.
- 3. HANDRAIL PANELS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. BOLTS USED ON HANDRAIL PLATE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153.
- 4. AFTER GALVANIZING, ALL ELEMENTS SHALL BE FREE OF FINS, ABRASIONS, ROUGH OR SHARP EDGES AND OTHER SURFACE DEFECTS.
- 5. HANDRAIL PANELS ON WALKWAY SHALL BE ERECTED PLUMB AND IN-LINE.

TRAINMAN'S WALK TIMBERS:

- 1. TRAINMAN'S WALK PLANKS SHALL BE COPPER NAPTHENATE TREATED DENSE SELECT NO. 1 OR BETTER DOUGLAS FIR-LARCH.
- 2. TIMBERS SHALL BE PLACED HEART SIDE DOWN TO PREVENT CUPPING.
- 3. APPROACH WALKWAY TIMBER STRINGERS AND MUD SILLS SHALL BE COPPER NAPTHENATE OR CREOSOTE TREATED DENSE SELECT NO. 1 OR BETTER DOUGLAS FIR-LARCH, ROUGH SAWN TO FULL NOMINAL DIMENSIONS.

RIVET REPLACEMENT NOTES:

- 1. CARE SHALL BE TAKEN NOT TO DAMAGE BASE METAL WHEN REMOVING RIVETS.
- 2. RIVETS NOTED TO BE REPLACED SHALL BE REPLACED WITH $\frac{7}{8}$ " DIAMETER ASTM A325 TYPE 3 HIGH STRENGTH BOLTS WITH HEAVY HEX NUT AND FLAT WASHER WITH $^{15}_{6}$ " DIAMETER OPEN HOLES UNLESS OTHERWISE NOTED.
- 3. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE "TURN-OF-THE-NUT METHOD" PER THE AREMA MANUAL FOR RAILWAY ENGINEERING, CHAPTER 15, PART 3. BOLTS SHALL ONLY BE USED ONCE IF FULLY TORQUED (DO NOT RE-USE).
- 4. ALL EXISTING TO NEW CONNECTION SURFACES SHALL HAVE THE MATING SURFACES CLEANED PER SSPC-SP3 OR EQUIVALENT.
- 5. ALL CLEANING BY-PRODUCTS SHALL BE COLLECTED AND DISPOSED OF IN ACCORDANCE WITH FEDERAL, STATE, LOCAL AND ARRC REQUIREMENTS.
- 6. RIVETS SHALL BE REMOVED AND REPLACED ONE AT A TIME AT EACH MEMBER CONNECTION, UNLESS OTHERWISE NOTED.

DRILL AND EPOXY GROUT NOTES FOR BEARING BLOCKS:

- 1. DRILL AND EPOXY GROUT ANCHOR RODS AND DOWELS USING APPROVED MULTI PART HIGH EARLY STRENGTH NON-SHRINK GROUT (5000 PSI MIN.).
- 2. DRILL HOLES 3/4" LARGER THAN THE NOMINAL ANCHOR ROD DIAMETER OR AS RECOMMENDED BY GROUT MANUFACTURER.
- 3. IMMEDIATELY BEFORE PLACING ANCHOR RODS, HOLES MUST BE CLEANED AND DRIED.
- 4. PLACE EPOXY GROUT INTO HOLES BY PRESSURE GROUTING AND INSERT ANCHOR RODS. IMMOBILIZE THE ANCHOR RODS UNTIL EPOXY GROUT REACHES STRENGTH.

POLYMER GROUT PAD NOTES:

- 1. POLYMER GROUT PAD SHALL CONSIST OF A 100% REACTIVE, RAPID SETTING, SOLVENT-FREE METHACRYLATE POLYMER CONCRETE SYSTEM. REQUIRED STRENGTH OF 2500 PSI MIN. PRIOR TO LOADING, 5000 PSI MIN. AT FULL CURE.
- 2. INSTALLATION SHALL BE PER THE MANUFACTURERS' RECOMMENDATIONS, INCLUDING EXTENDING FOR THICKER BEARING
- 3. PROVIDE FULL BEARING AND BONDING TO PRECAST CONCRETE BEARING BLOCKS. POUR PAD TO ELEVATION AND ROUGHEN POLYMER GROUT PAD FOR EPOXY ADHESIVE PRIOR TO SETTING PRECAST BLOCK.

STEEL BEARING NOTES:

- 1. STEEL BEARINGS SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH THE AREMA MANUAL FOR RAILWAY ENGINEERING (MRE), CHAPTER 15: STEEL STRUCTURES AND PROJECT SPECIFICATIONS.
- 2. FINISHED SURFACES SHALL BE SHOP COATED WITH AN APPLICATION OF WATERPROOF MULTIPURPOSE GREASE (NATIONAL LUBRICATING GREASE INSTITUTE (NGLI) NO. 3) IMMEDIATELY FOLLOWING THE SURFACING PROCESS. PRIOR TO BEARING INSTALLATION, THE SHOP-COATED SURFACE SHALL BE WIPED CLEAN.
- 3. SLIDING TYPE EXPANSION BEARINGS SHALL BE COMPRISED OF PTFE AND STAINLESS STEEL CONFORMING TO THE REQUIREMENTS OF AREMA MRE CHAPTER 15, PARTS 5.5, 5.11, AND PROJECT SPECIFICATIONS.
- 4. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND VISIBLE AFTER BEARING INSTALLATION.
- 5. BEARINGS SHALL BE ACCURATELY SET TO ACHIEVE PROPER ALIGNMENT, LEVEL & FULL UNIFORM BEARING. SETTING OF ROCKERS, ROLLERS AND SLIDING SURFACES SHALL TAKE INTO ACCOUNT ANY VARIATION FROM MEAN TEMPERATURE OF THE SUPPORTED SPAN AT THE TIME OF SETTING AND ANY OTHER ANTICIPATED CHANGES IN LENGTH OF THE SUPPORTED SPAN. CARE SHALL BE TAKEN THAT THE FULL AND FREE MOVEMENT OF THE SUPERSTRUCTURE AT MOVABLE BEARINGS IS NOT RESTRICTED BY IMPROPER SETTING OR ADJUSTMENTS OF THE BEARING.
- 6. ALL WELDED ASSEMBLIES SHALL BE STRESS RELIEVED AFTER FABRICATION IN ACCORDANCE WITH THE CURRENT EDITION OF THE BRIDGE WELDING CODE, AWS DI.5.

CONCRETE REPAIR NOTES:

- 1. INTENTIONALLY ROUGHEN SURFACE OF EXISTING CONCRETE TO 1/4" AMPLITUDE BY CHIPPING AND REMOVING POOR CONCRETE TO SOUND CONCRETE. COAT SURFACE WITH EPOXY BONDING AGENT PRIOR TO CASTING NEW CONCRETE PER MANUFACTURER'S RECOMMENDATIONS.
- 2. MATERIAL AND WORKMANSHIP SHALL BE PER THE CURRENT ARRC STANDARD SPECIFICATIONS.
- 3. CAST-IN-PLACE CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, UNLESS OTHERWISE NOTED IN SPECIFICATION FOR CONCRETE CONSTRUCTION.
- 4. CONCRETE SHALL BE VIBRATED INTERNALLY DURING PLACEMENT TO PROVIDE A THOROUGH CONSOLIDATION AND COMPACTION. CARE SHALL BE TAKEN TO AVOID DISPLACEMENT OF EMBEDDED ITEMS.
- 5. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. REINFORCING STEEL SHALL HAVE A MINIMUM 2" CLEAR COVER UNLESS OTHERWISE SHOWN OR NOTED.
- 6. DOWEL BAR BENDING AND STANDARD HOOK DIMENSIONS SHALL BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE" AS PUBLISHED BY THE CONCRETE REINFORCING STEEL INSTITUTE UNLESS OTHERWISE SHOWN OR NOTED.
- 7. CHAMFER EXPOSED CORNERS $\frac{3}{4}$ ", TYPICAL.
- 8. DOWEL BARS SHALL BE DOWELED INTO EXISTING CONCRETE USING AN APPROVED EPOXY. DRILL BIT DIAMETER AND INSTALLATION PROCEDURE SHALL BE IN ACCORDANCE WITH POWERS AC100+ GOLD EPOXY RECOMMENDATIONS TO THE LENGTH INDICATED ON THESE PLANS. USE OF ALTERNATE EPOXY MAY REQUIRE ADJUSTMENTS TO PLANS.
- 9. WHERE SAW CUTTING OF EXISTING CONCRETE BACKWALL IS REQUIRED, CUTS SHALL BE MADE USING A WIRE SAW IN NEAT VERTICAL OR HORIZONTAL LINES. SEAL CUT SURFACES WITH EPOXY SEALANT PER THE PROJECT SPECIFICATIONS.

TRUSS REHAB STEEL NOTES:

- 1. FABRICATION AND WORKMANSHIP SHALL CONFORM TO CURRENT AREMA MANUAL FOR RAILWAY ENGINEERING, CHAPTER 15, STEEL STRUCTURES AND PROJECT SPECIFICATIONS.
- 2. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM DESIGNATION A709 GRADE 50 EXCEPT AS NOTED.
- 3. ANY BOLTS FOR TEMPORARY FIT THAT ARE FULLY-STRESSED, SHALL NOT BE RE-USED.
- 4. PAINT ALL NON-WEATHERING, NON-GALVANIZED STRUCTURAL STEEL IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. SHOP PAINT ALL NEW STEEL MEMBERS DESIGNATED TO BE PAINTED.
- 5. NDT TESTING OF TRUSS PLATES AT ALL REMOVED WALKWAY BRACE LOCATION WELDS IS REQUIRED AND SHALL BE PERFORMED PER THE PROJECT SPECIFICATIONS TO ENSURE STRUCTURAL INTEGRITY OF TRUSS MEMBERS. TESTING RESULTS SHALL BE SUBMITTED TO ARRC FOR REVIEW AND APPROVAL.
- 6. STENCIL PIECE MARK ON EACH SHOP FABRICATED COMPONENT.

CONCRETE RISER BLOCK NOTES:

- 1. FABRICATION AND WORKMANSHIP SHALL CONFORM TO CURRENT AREMA MANUAL FOR RAILWAY ENGINEERING, CHAPTER 8, CONCRETE STRUCTURES AND PROJECT SPECIFICATIONS.
- 2. STENCIL PIECE MARK, DATE OF FABRICATION AND LIFTING WEIGHT ON EACH PIECE.
- 3. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS, UNLESS OTHERWISE NOTED.
- 4. CONCRETE SHALL BE VIBRATED INTERNALLY DURING PLACEMENT TO PROVIDE THOROUGH CONSOLIDATION AND COMPACTION. CARE SHALL BE TAKEN TO AVOID DISPLACEMENT OF EMBEDDED ITEMS.
- 5. LIFTING INSERTS ARE TO BE DOUBLE FLARED COIL LOOP INSERTS, CONTRACTOR IS RESPONSIBLE FOR ADEQUACY OF LIFTING INSERTS WITH A 4 TO 1 SAFETY FACTOR. FILL INSERTS WITH GROUT AFTER PLACING BLOCKS.
- 6. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM DESIGNATION A709 GRADE 50W EXCEPT AS NOTED. WELDED REINFORCING STEEL SHALL CONFORM TO ASTM A706, GRADE 60
- 7. WELDING SHALL CONFORM TO CURRENT A.W.S. BRIDGE WELDING CODE D1.5. ALL WELDING TO BE HEAT CONTROLLED TO PREVENT WARPING AND PROVIDE FULL, EVEN BEARING AT BASE AND TOP SURFACES.

WALKWAY BOLLARD NOTES:

- 1. STEEL PIPE SHALL HAVE A MINIMUM TENSILE STRENGTH OF 36,000 P.S.I. (ASTM GRADE A).
- 2. BOLLARDS SHALL BE 4" DIA. DUCTILE IRON STEEL PIPE WITH A WALL THICKNESS OF 0.188".
- 3. CONCRETE FOR FOUNDATIONS SHALL HAVE A MINIMUM STRENGTH OF 3,000 P.S.I. AFTER 28 DAYS.
- 4. BOLLARDS SHALL BE PAINTED WITH A PRIME COAT (GRAY ORGANIC ZINC-RICH EPOXY PRIMER) AND A TOP COAT OF SAFETY YELLOW LATEX.

SEISMIC DESIGN NOTES:

- 1. SEISMIC DESIGN PER AREMA CHAPTER 9 FOR NEW TRUSS BEARING ASSEMBLIES, ANCHORAGES, AND RISER BLOCKS ONLY.
- 2. SEISMIC DESIGN PARAMETERS:

Α.	SITE	CLASS:		D	(ASSUMED)

- B. IMMEDIATE SAFETY FACTOR:
- C. IMMEDIATE VALUE FACTOR:
- 3. SEISMIC DESIGN LIMIT STATES:

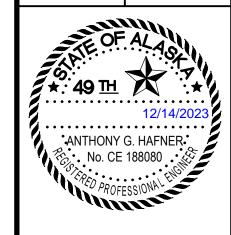
D. REPLACEMENT VALUE:

LIMIT STATE	IMPORTANCE FACTOR	RETURN PERIOD		ACCEL. AD FOR SITE,		SITE CLASS ADJUSTED PGA (g)
			F _{PGA}	F _A	F _V	PGA _M
SERVICEABILITY	3.6	95	1.397	1.474	2.258	0.28
ULTIMATE	2.4	365	1.160	1.239	1.909	0.39
SURVIVABILITY	3.6	2328	1.000	1.000	1.529	0.60

4. SEISMIC ACCELERATION RESPONSE SPECTRA:

	AREMA SEISMIC RESPONSE COEFFICIENTS (C _M)						
PERIOD		RETURN PERIOD					
(SECONDS)	95-YEAR	365-YEAR	2328-YEAR				
	SERVICEABILITY	ULTIMATE	SURVIVABILITY				
0	0.600	0.869	1.253				
0.1	0.600	0.869	1.253				
0.2	0.600	0.869	1.253				
0.3	0.600	0.869	1.253				
0.4	0.600	0.869	1.253				
0.5	0.600	0.869	1.253				
0.6	0.510	0.781	1.200				
0.7	0.437	0.669	1.029				
0.8	0.382	0.586	0.900				
0.9	0.340	0.521	0.800				
1.0	0.306	0.469	0.720				
1.2	0.255	0.390	0.600				
1.4	0.218	0.335	0.514				
1.6	0.191	0.293	0.450				
1.8	0.170	0.260	0.400				
2.0	0.153	0.234	0.360				

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BRIDGE

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CONSTRUCTION WORK SEQUENCE:

THE FOLLOWING IS SUGGESTED CONSTRUCTION SEQUENCE, FINAL PHASING PLANS TO BE DETERMINED BY CONTRACTOR.

- 1. CONSTRUCT TEMPORARY ACCESS FROM STAGING AREA TO BRIDGE SITE.
- 2. RELOCATE AND TEMPORARILY SUPPORT EXISTING UTILITY AND CONDUIT OFF BRIDGE TRAINMAN'S WALK.
- 3. PLACE PEDESTRIAN WARNING SIGNS AND TRAIL CLOSURE. PLACE WARNING SIGNS ON RIVER NOTING OVERHEAD WORK AS BEING PERFORMED AS REQUIRED.
- 4. DURING TRACK WORK WINDOWS, COMPLETE THE FOLLOWING (NOTE: WALKWAY REPLACEMENT AND TRUSS REHABILITATION WORK MAY BE PERFORMED SIMULTANEOUSLY):

WALKWAY REPLACEMENT

- a. ATTACH WALKWAY SUPPORT ANGLE BRACKETS TO EXISTING TRUSS AT PRIMARY NODE LOCATIONS.
- b. CONSTRUCT APPROACH WALKWAY
 - 1. REMOVE WALKWAY AT APPROACHES AND FENCING.
 - 2. INSTALL STEEL BRACKETS ON BACKWALL AND REMOVE EXISTING CONCRETE FROM BACKWALL AS REQUIRED TO ALLOW TIMBER WALKWAY STRINGERS TO PASS THROUGH.

CONSTRUCTION WORK SEQUENCE (CONTINUED):

- 3. INSTALL NEW TREATED TIMBER MUD SILLS DIRECTLY BEHIND EXISTING MUD SILLS.
- 4. REMOVE EXISTING TIMBER SUPPORT BENT ON SOUTH APPROACH, CUT TIMBER CAP AT FACE OF EXISTING GROUND LINE (DO NOT REMOVE EMBEDDED PORTION OF BEAM).
- 5. INSTALL WALKWAY TIMBER STRINGERS AT APPROACHES AND CONNECTION PLATES.
- 6. ATTACH HANDRAIL AT APPROACHES.
- c. REMOVE EXISTING TRAINMAN'S WALK ON BRIDGE. GRIND WELDED CONNECTIONS WITHOUT DAMAGING TRUSS MEMBER BASE METAL TO REMAIN.
- d. PERFORM NDT ULTRASONIC TESTING OF EXISTING TRAINMAN'S WALK SUPPORT WELDS ON TRUSS LOW CHORD, VERTICALS, AND DIAGONALS PER PROJECT SPECIFICATIONS.
- e. ATTACH NEW TRAINMAN'S WALK SUPPORT BRACKETS TO EXISTING TRUSS AT PRIMARY NODE LOCATIONS.
- f. PREDRILL HOLES AND PREPARE MATING SURFACES. ATTACH TO TRAINMAN'S WALK SUPPORT BEAMS, CROSSBEAMS, TIMBER TRAINMAN'S WALK PLANKS, AND HANDRAIL.
- g. ERECT PERMANENT CHAIN LINK FENCE AT BOTH APPROACHES.

CONSTRUCTION WORK SEQUENCE (CONTINUED):

- TRUSS REHABILITATION
- a. REMOVE AND REPLACE STRINGER AND END FLOORBEAM BOTTOM FLANGE ANGLES AFTER REMOVING EXISTING TRAINMAN'S WALK. SEE STRINGER AND END FLOORBEAM WORK SEQUENCE NOTES THIS SHEET.
- b. REPAIR CONCRETE SURFACE AT ABUTMENT 1. JACK AND REPLACE TRUSS BEARINGS.
- c. ADD COVER PLATES TO TRUSS DIAGONALS AND POSTS AS INDICATED ON PLANS. SEE TRUSS STRENGTHENING SEQUENCES ON SHEET 11.
- d. REPLACE RIVETS WITH HIGH STRENGTH BOLTS ON TRUSS POSTS, DIAGONALS, AND BOTTOM CHORD AS INDICATED ON PLANS.
- 5. REMOVE TEMPORARY WORKS AND RESTORE SITE TO PRE-CONSTRUCTION CONDITIONS.
- 6. WHEN WEATHER CONDITIONS ALLOW, COMPLETE FIELD CLEANING AND PAINTING OF CONNECTIONS AND AREAS WITH EXPOSED NON-WEATHERING, NON-GALVANIZED STEEL SURFACES.
- 7. PLACE TOPSOIL AND SEED PREVIOUSLY VEGETATED DISTURBED AREAS.

END FLOORBEAM AND STRINGER STRENGTHENING WORK SEQUENCE:

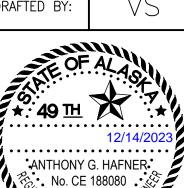
OPTION 1 — FULL REPLACEMENT DURING LONG WORK WINDOWS.

- 1. REMOVE RIVETS IN EXISTING BOTTOM FLANGE ANGLES.
- 2. REPLACE BOTTOM FLANGE ANGLES AND INSTALL H.S. BOLTS.

OPTION 2 — REPLACEMENT DURING SHORT WORK WINDOWS.

- 1. REMOVE RIVETS IN EXISTING BOTTOM FLANGE ANGLES AND REPLACE RIVETS WITH TEMPORARY H.S. BOLTS.
- 2. DURING FINAL WORK WINDOW REMOVE ALL TEMPORARY H.S. BOLTS (TEMPORARY H.S. BOLTS MAY BE RE-USED FOR FUTURE TEMPORARY FLANGE CONNECTIONS IN STEP 1). REPLACE BOTTOM FLANGE ANGLES WITH NEW ANGLES AND INSTALL NEW H.S. BOLTS USING TURN-OF-THE-NUT METHOD.

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CAPITAL PROJE

RAILROAD

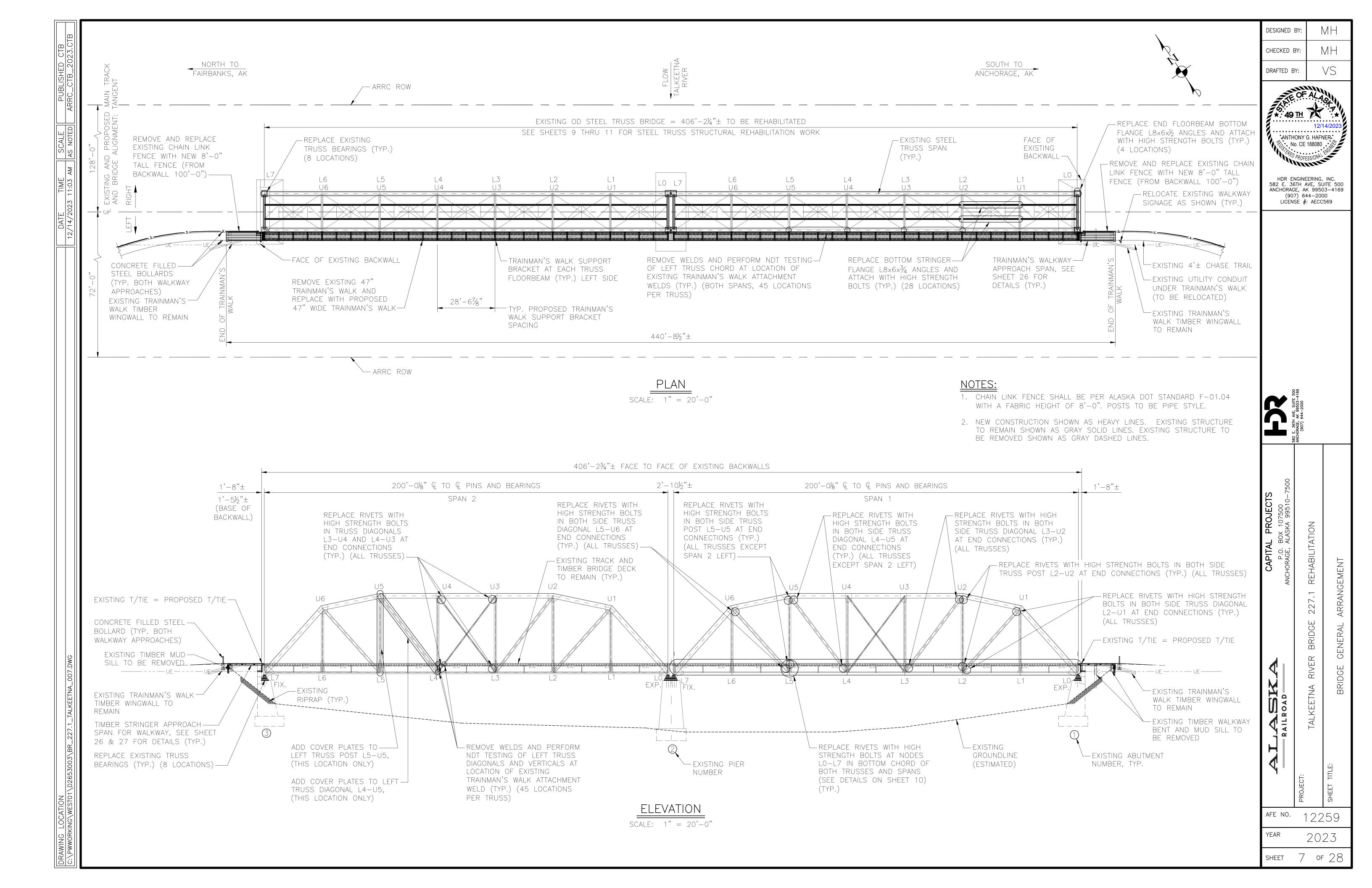
RAILROAD

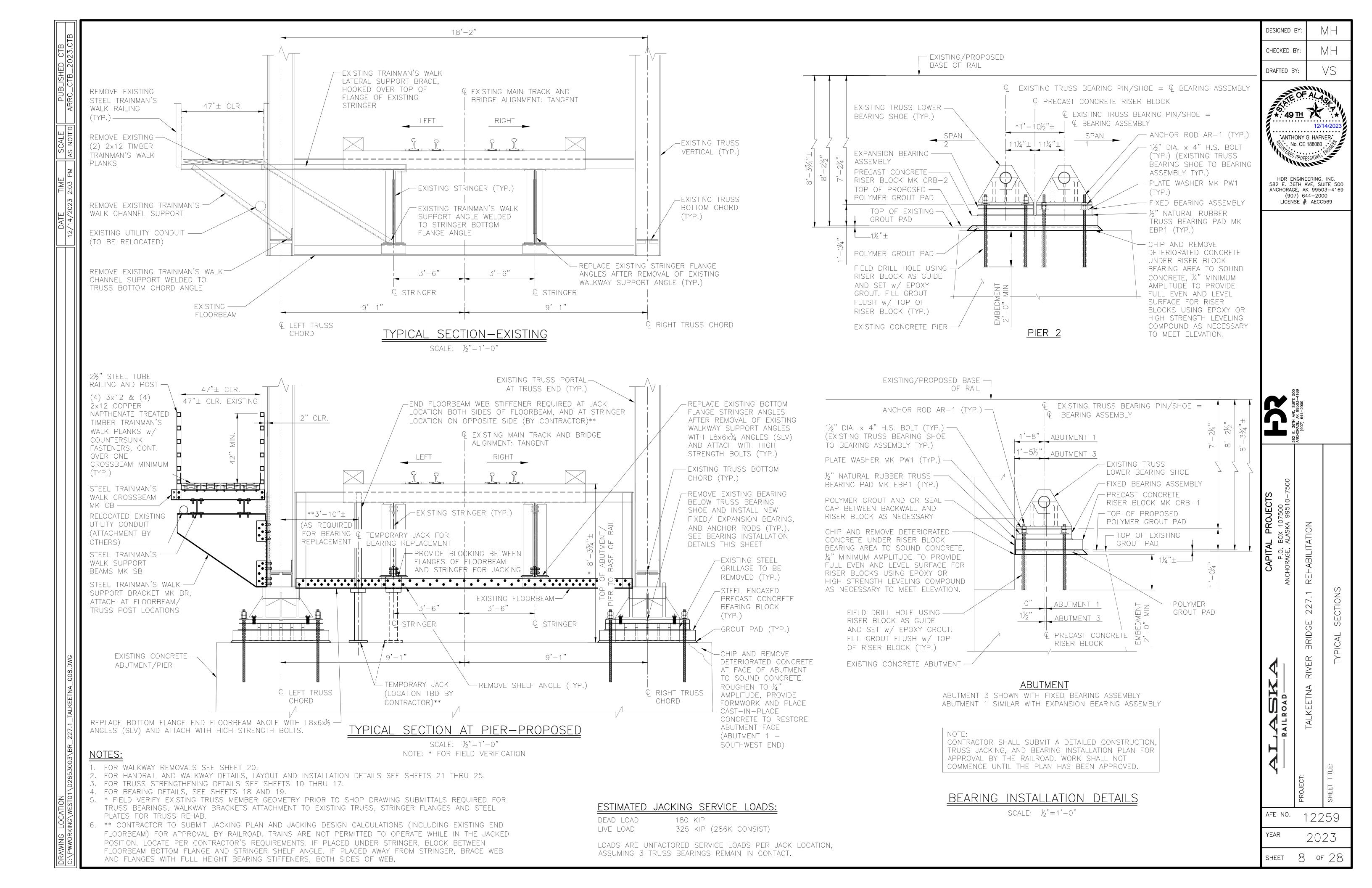
TALKEETNA RIVER BRIDGE 227.1 REHABILITATION

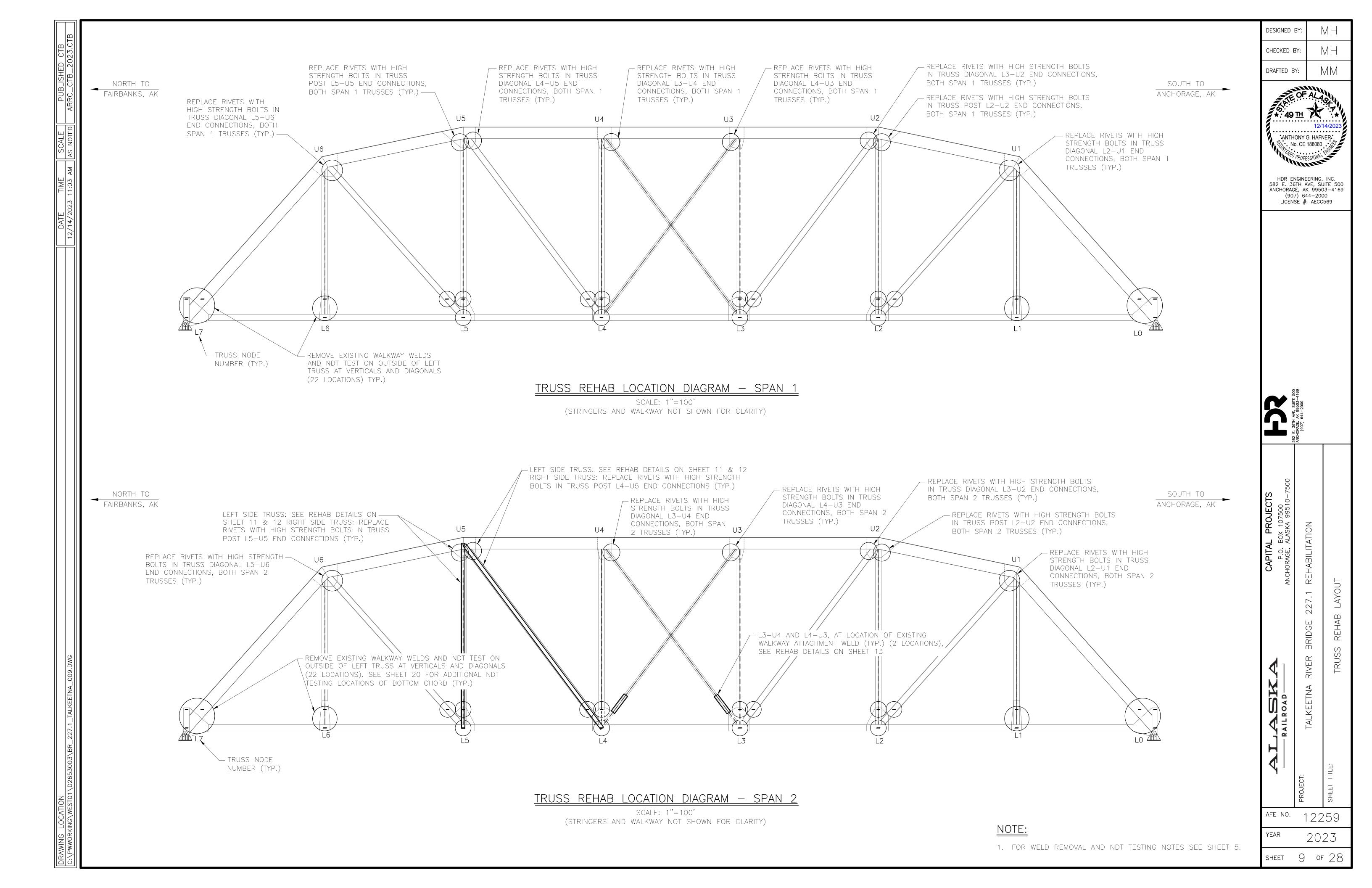
CONSTRUCTION WORK SEQUENCE AND GENERAL SITE PL

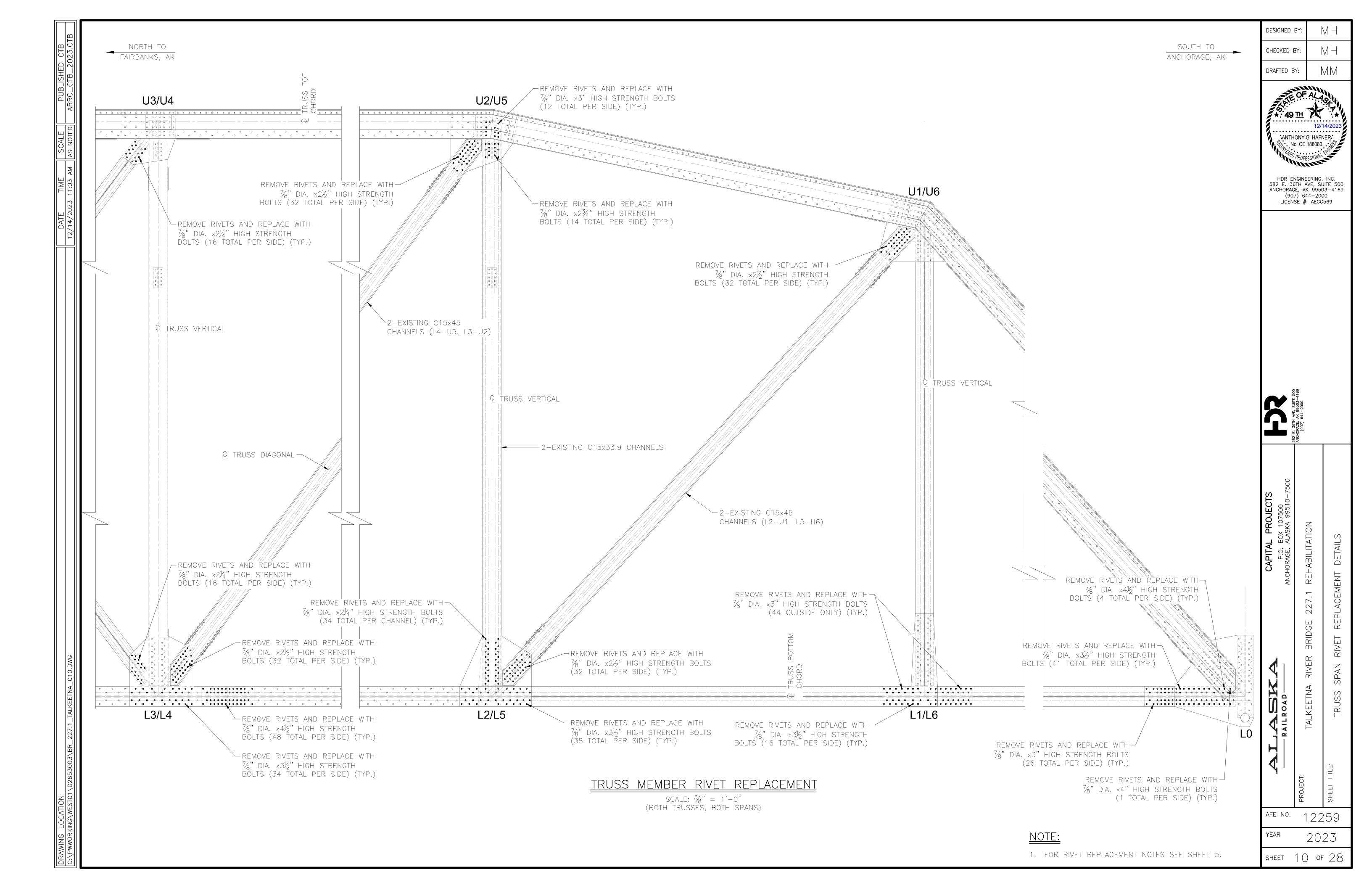
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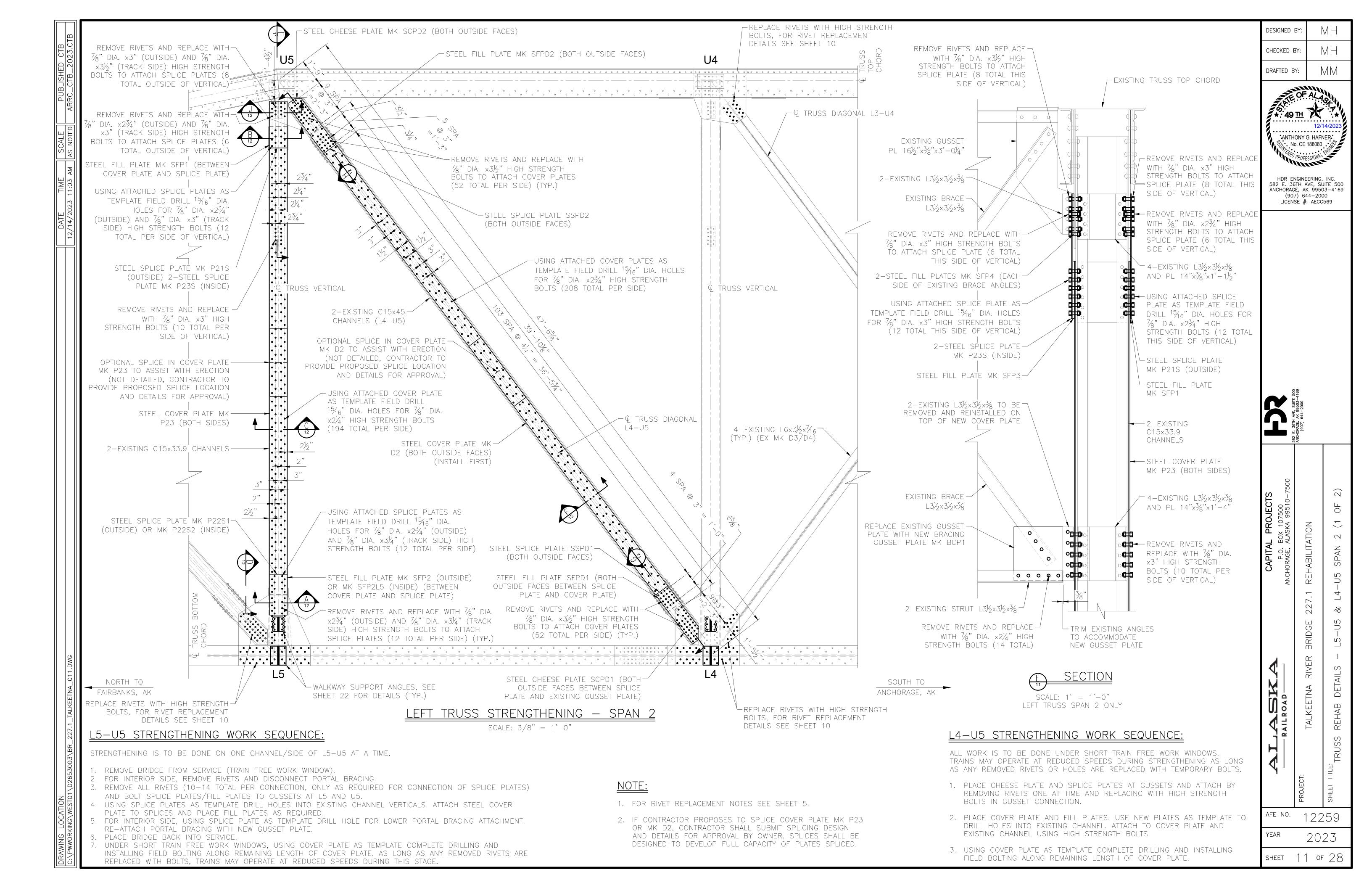
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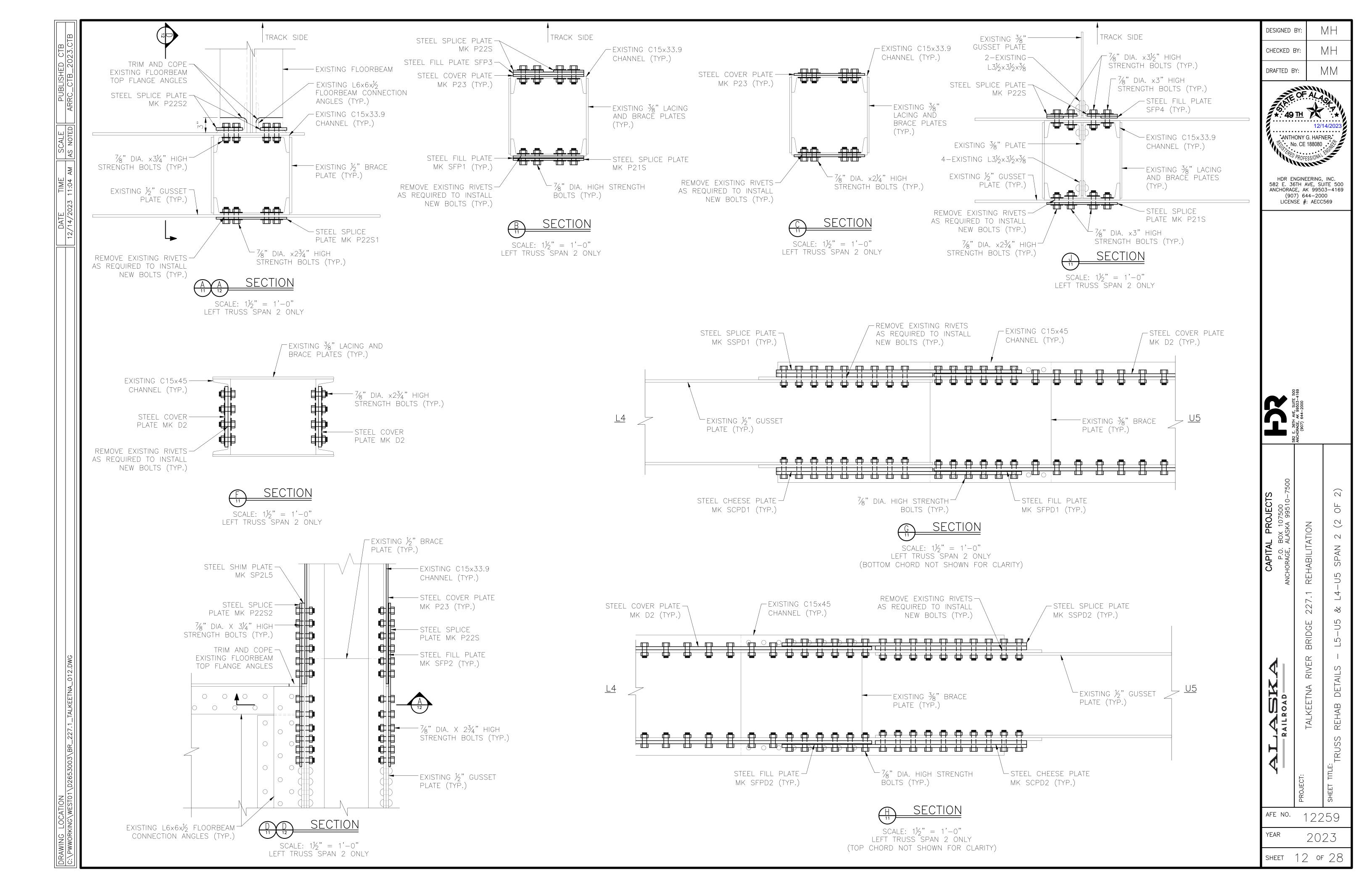


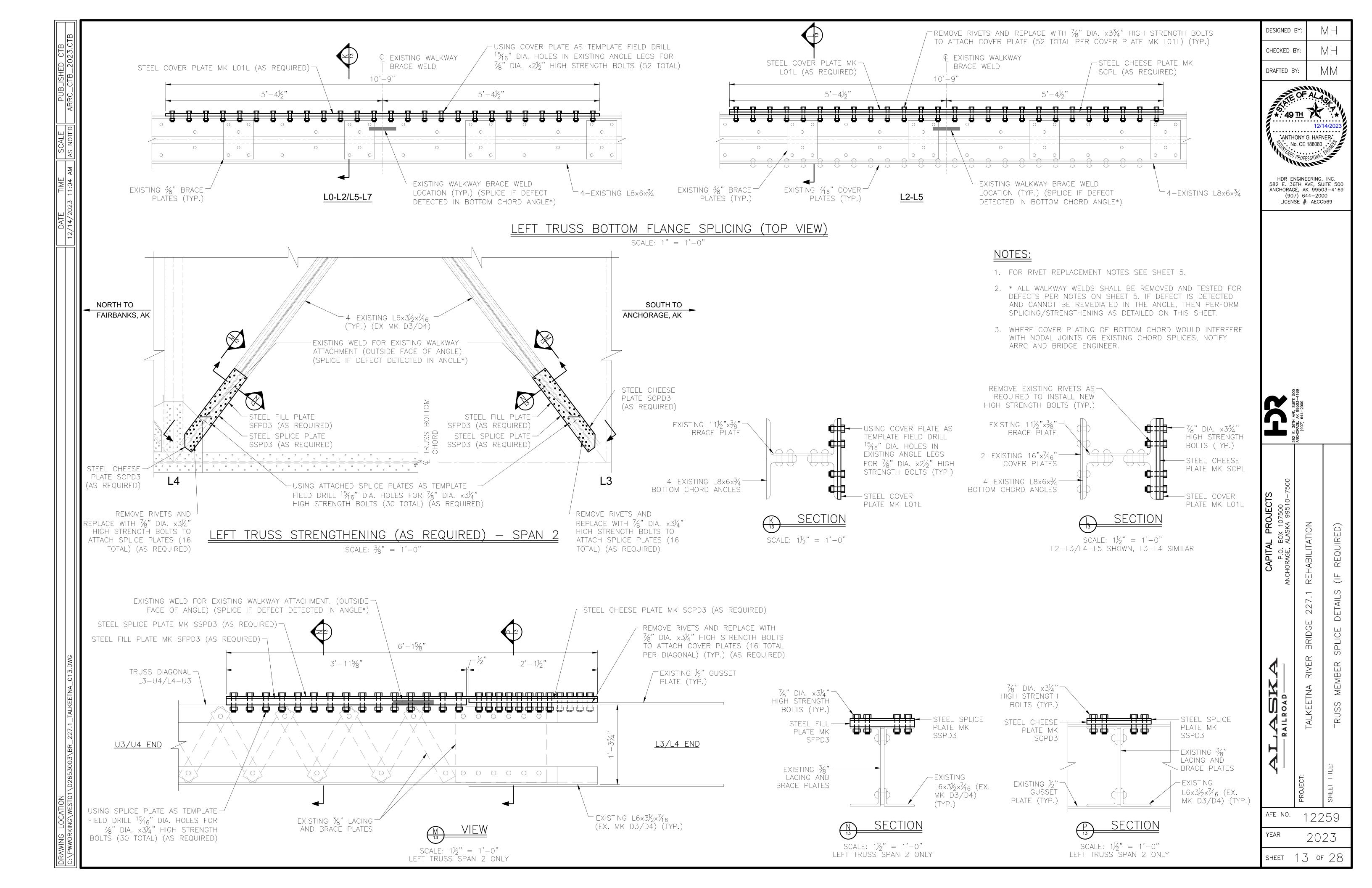


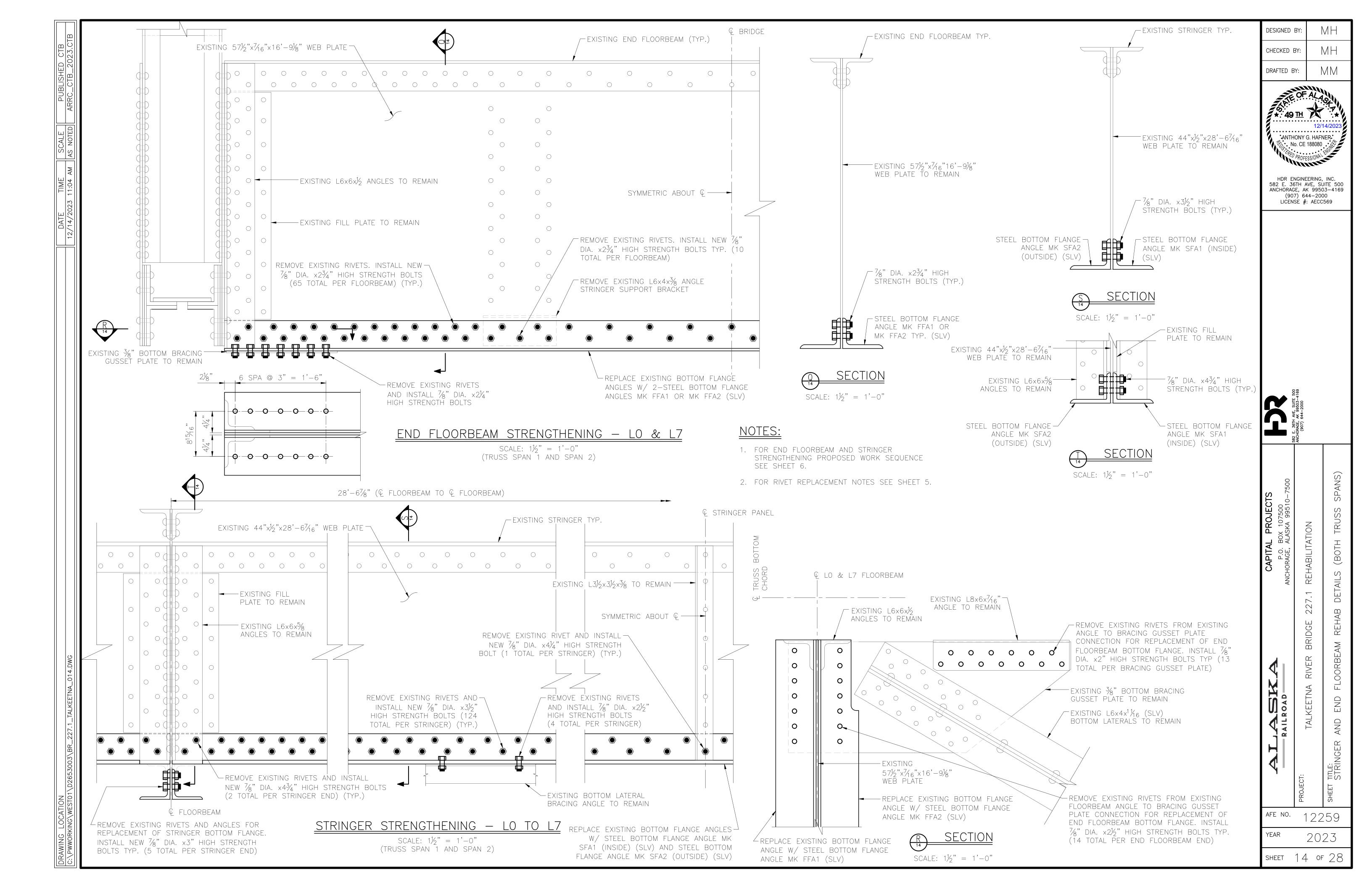


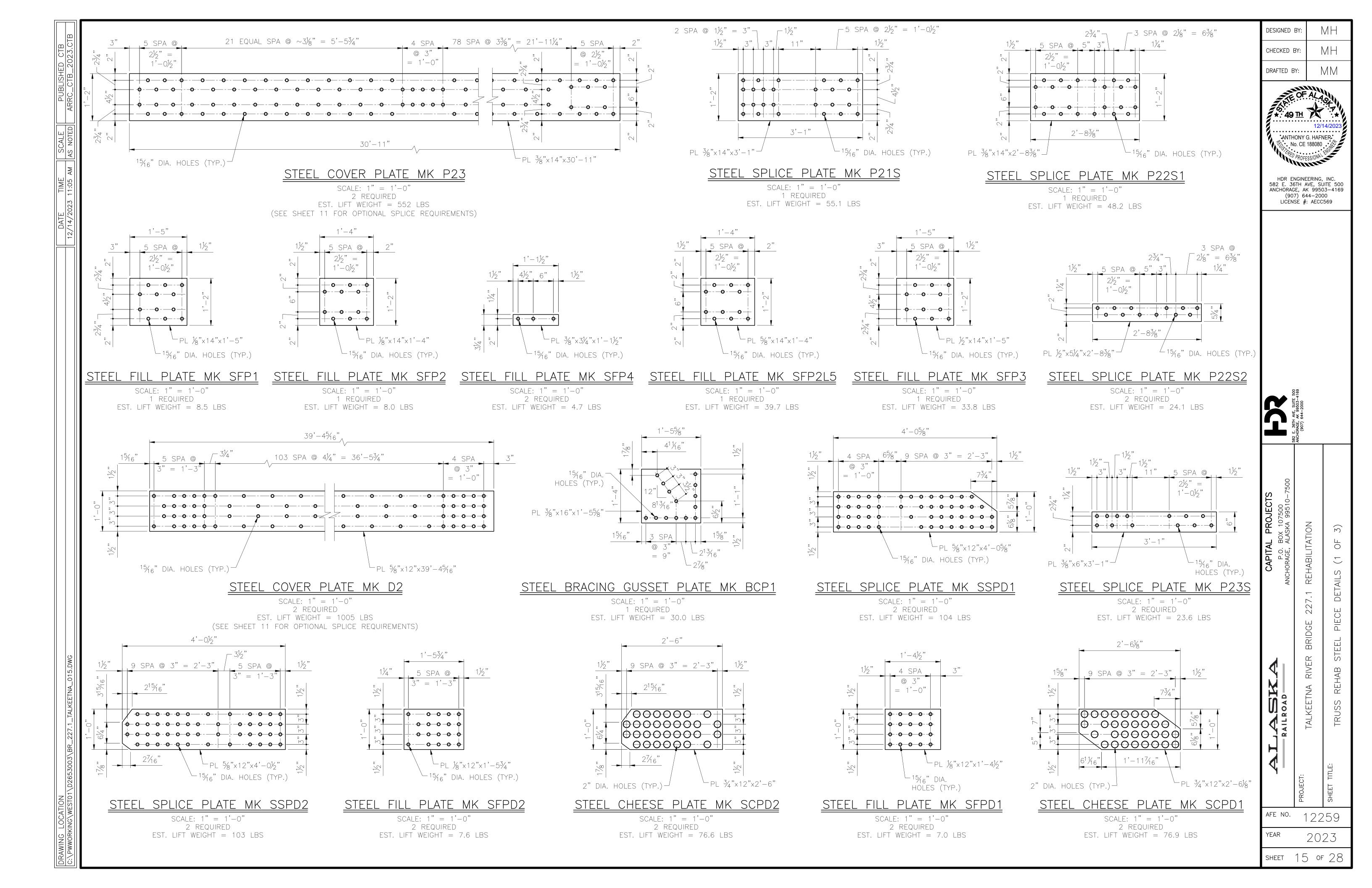


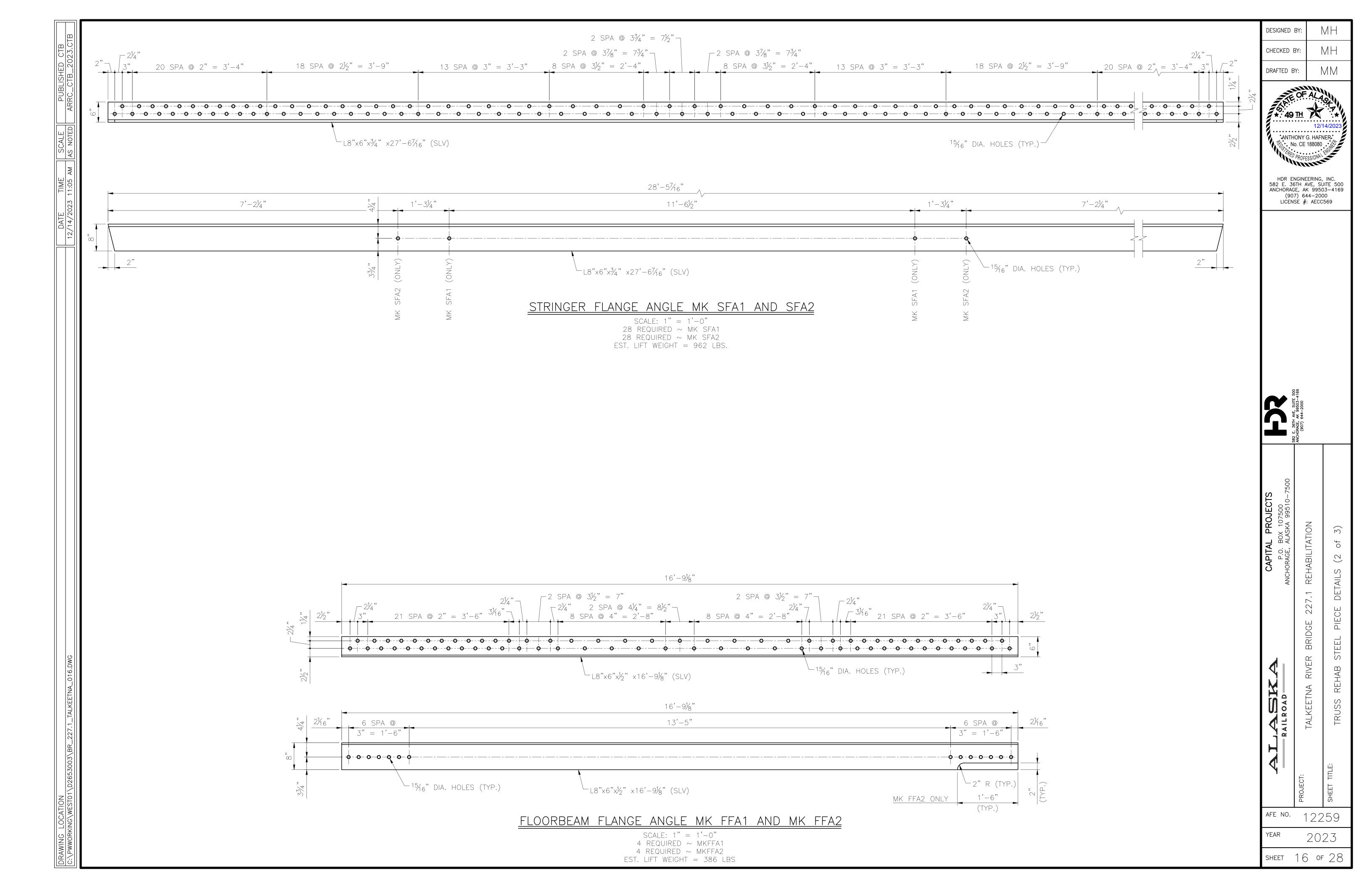


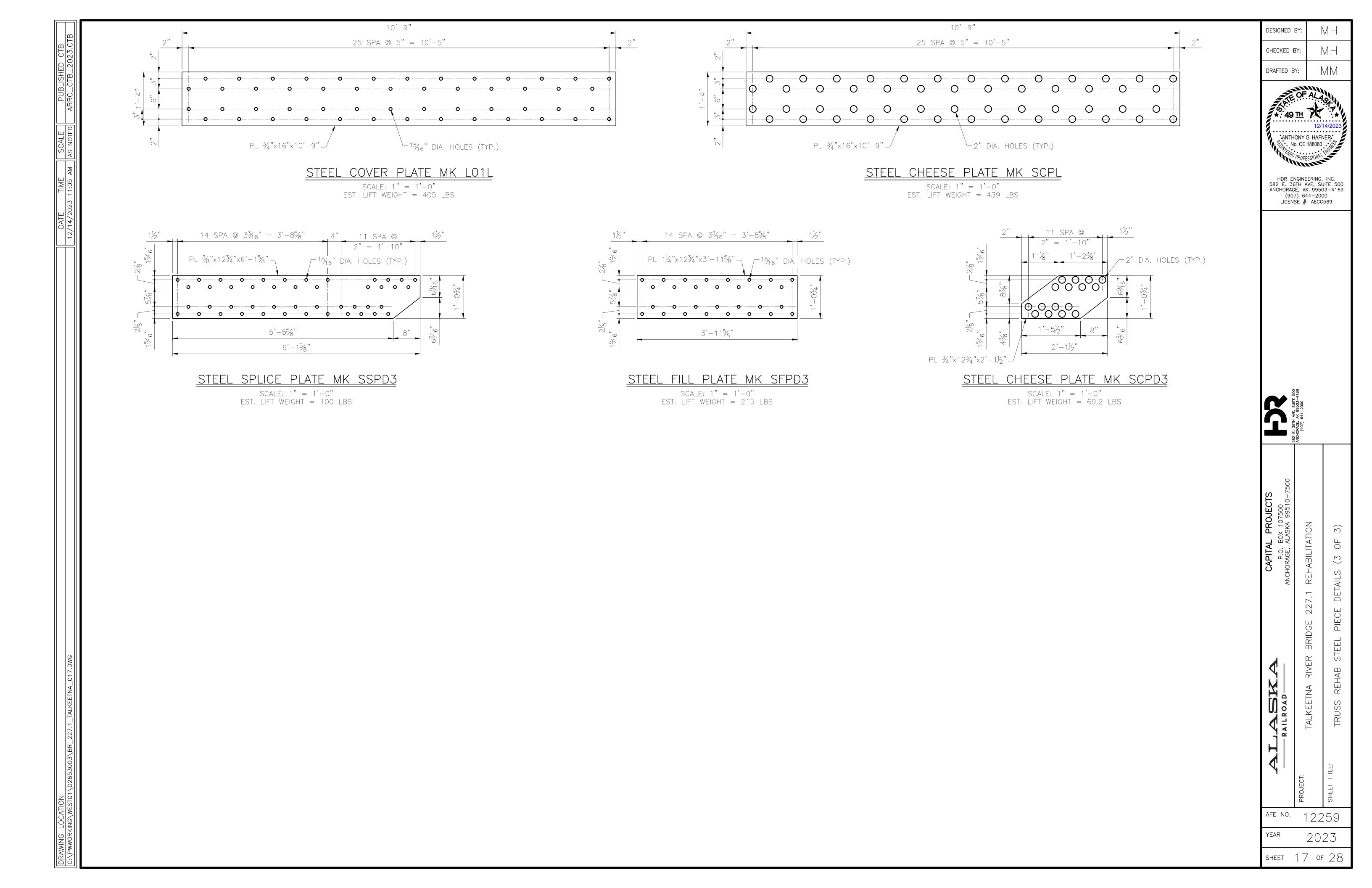


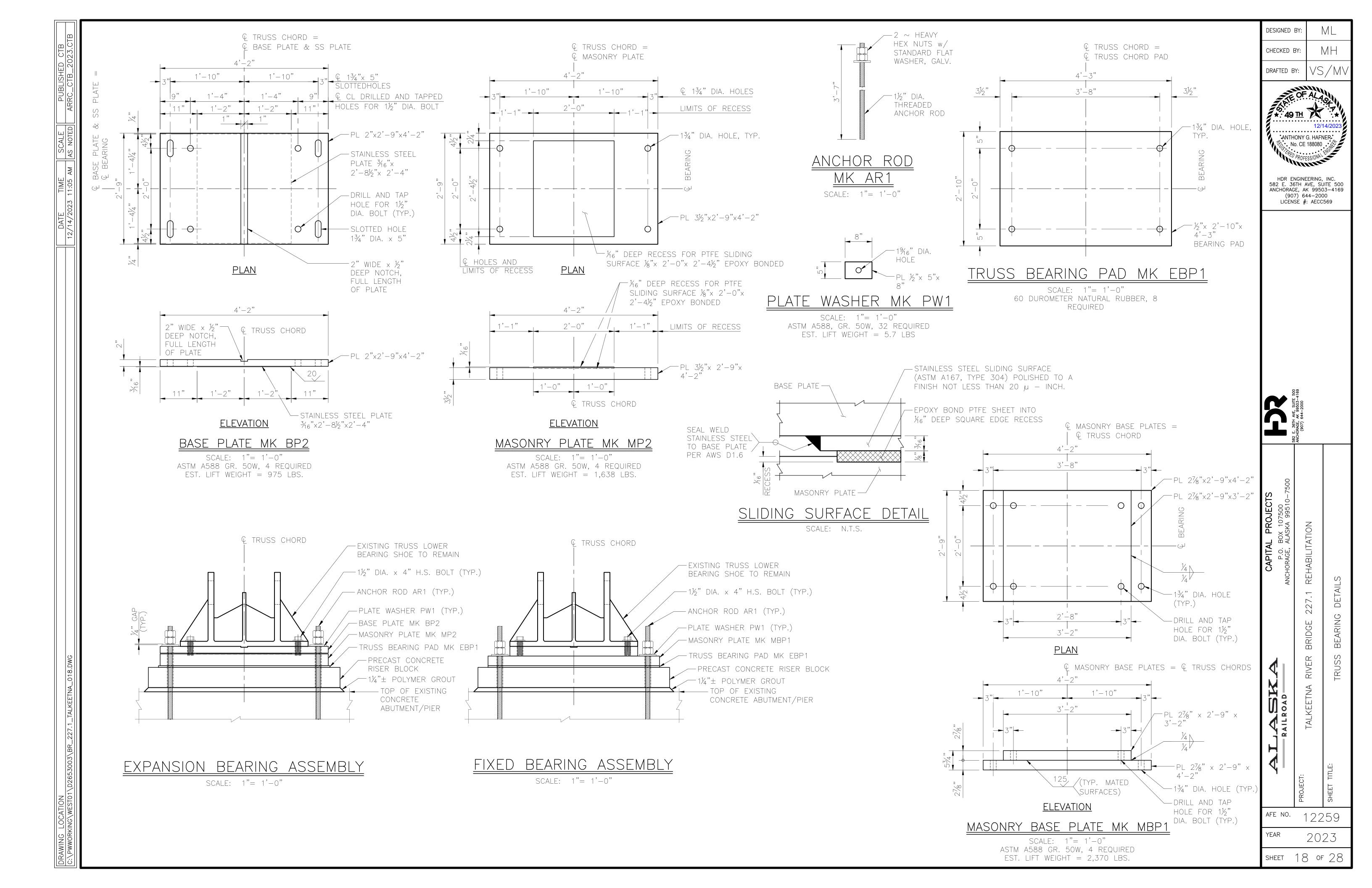


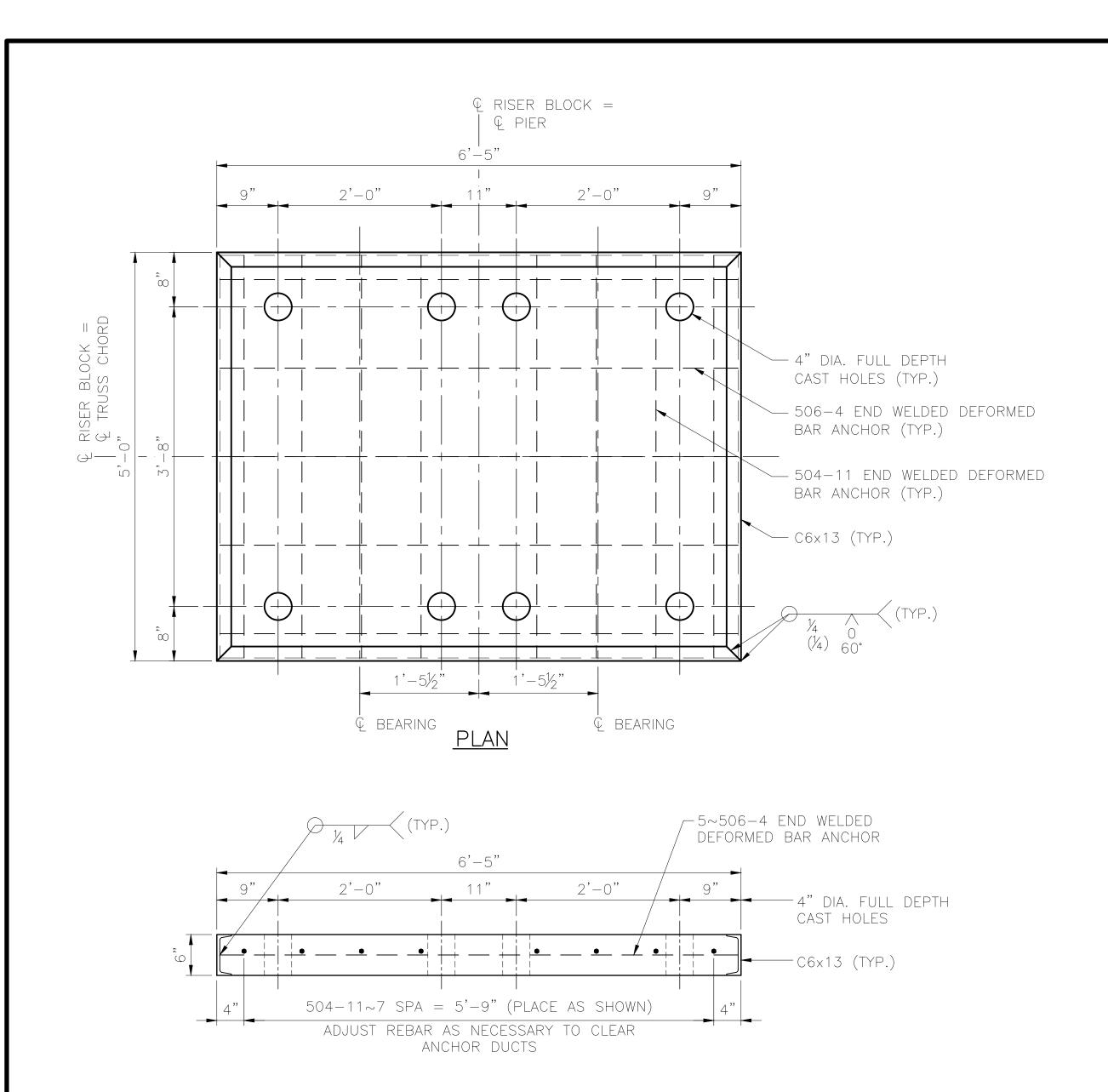




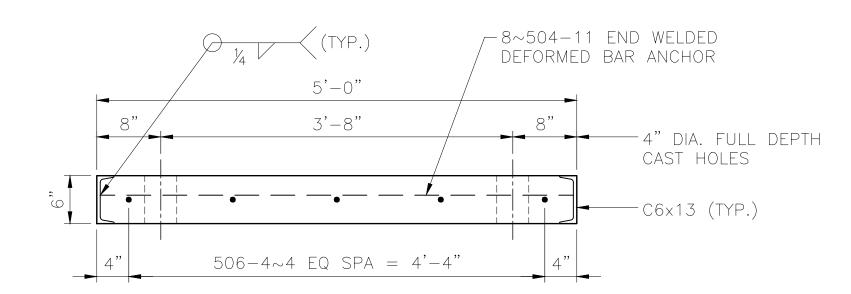








ELEVATION

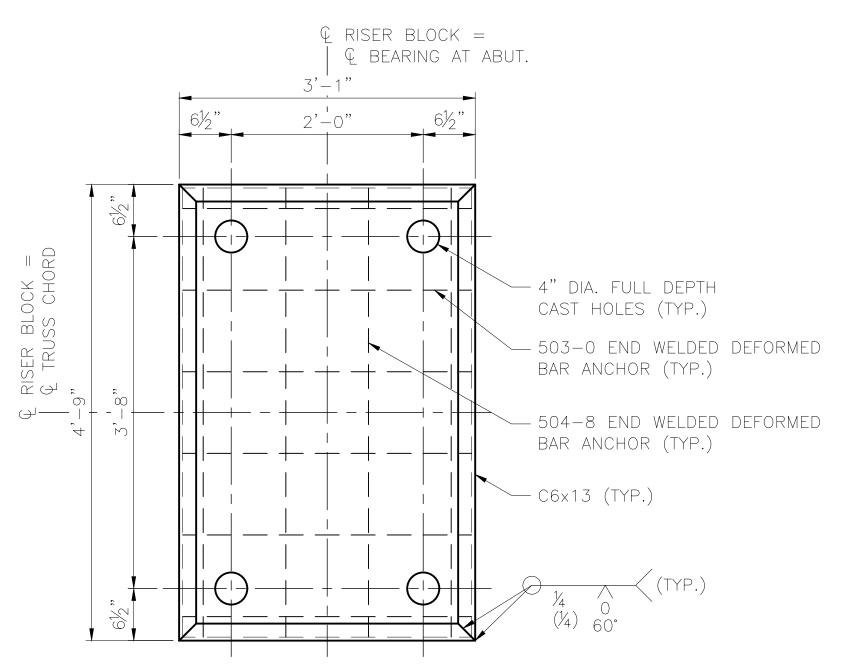


<u>SIDE</u>

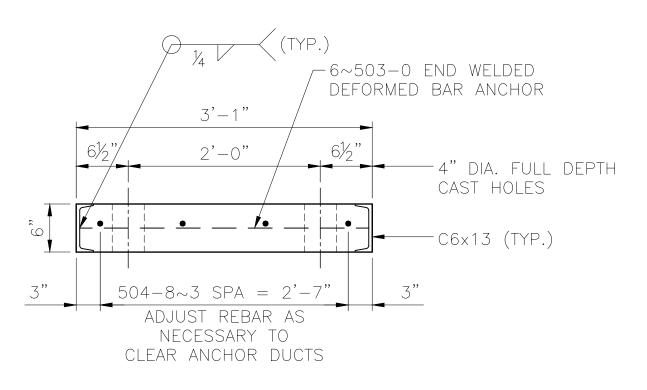
PRECAST CONCRETE RISER BLOCK CRB2

SCALE: 1" = 1'-0"PIER 2

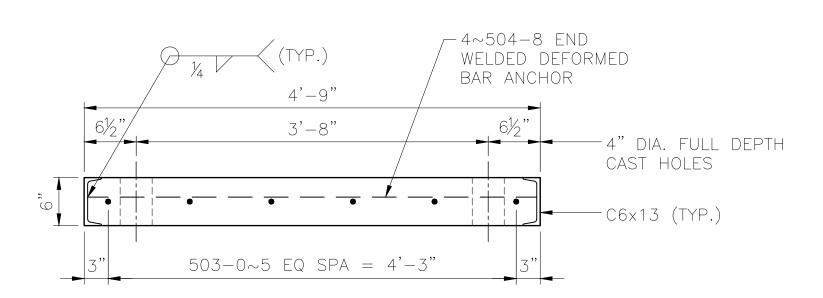
2 REQUIRED EST. LIFT WEIGHT = 2,705 LBS



<u>PLAN</u>



ELEVATION



<u>SIDE</u>

PRECAST CONCRETE RISER BLOCK CRB1

SCALE: 1" = 1'-0"

ABUTMENT 1 & 3

4 REQUIRED EST. LIFT WEIGHT = 1,305 LBS

LIST (OF RE	EINFO	RCING
BARS	FOR	MK	CRB2

QTY	MARK	SIZE	TYPE	LENGTH
8	504-11	5	STR	4'-11"
5	506-4	5	STR	6'-4"
23 LF	23 LF C6×13 (ASTM A709, GR 50W)			

83 LBS = WEIGHT OF REINFORCING STEEL, ASTM A706, GR60 0.60 CY = TOTAL VOLUME OF CLASS A CONCRETE

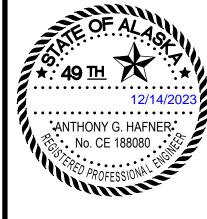
LIST OF REINFORCING BARS FOR MK CRB1

QTY	MARK	SIZE	TYPE	LENGTH
4	504-8	5	STR	4'-8"
6	503-0	5	STR	3'-0"
16 LF	C6×13 (ASTM	A709, GR	50W)	

39 LBS = WEIGHT OF REINFORCING STEEL, ASTM A706, GR60 0.28 CY = TOTAL VOLUME OF CLASS A CONCRETE

CHECKED BY: DRAFTED BY:

DESIGNED BY:



HDR ENGINEERING, INC. 582 E. 36TH AVE, SUITE 500 ANCHORAGE, AK 99503-4169 (907) 644-2000 LICENSE #: AECC569

227.1

AFE NO. 12259 2023

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